FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION

Division of Environmental Assessment and Restoration, Bureau of Watershed Restoration

NORTHEAST DISTRICT • LOWER ST. JOHNS BASIN

Final TMDL Report

Nutrient TMDL for Arlington River (WBID 2265A)

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October 2009

Acknowledgments

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Websites

Florida Department of Environmental Protection, Bureau of Watershed Management

TMDL Program http://www.dep.state.fl.us/water/tmdl/index.htm Identification of Impaired Surface Waters Rule http://www.dep.state.fl.us/legal/Rules/shared/62-303/62-303.pdf STORET Program http://www.dep.state.fl.us/water/storet/index.htm 2008 305(b) Report http://www.dep.state.fl.us/water/docs/2008 Integrated Report.pdf Criteria for Surface Water Quality Classifications http://www.dep.state.fl.us/water/wqssp/classes.htm Basin Status Report for the Lower St. Johns Basin http://www.dep.state.fl.us/water/basin411/sj_lower/status.htm Water Quality Assessment Report for the Lower St. Johns Basin http://www.dep.state.fl.us/water/basin411/sj_lower/assessment.htm

U.S. Environmental Protection Agency, National STORET Program

Region 4: Total Maximum Daily Loads in Florida http://www.epa.gov/region4/water/tmdl/florida/ National STORET Program http://www.epa.gov/storet/

Chapter 1: INTRODUCTION

1.1 Purpose of Report

This report presents the Total Maximum Daily Load (TMDL) for nutrients for the Arlington River in the North Mainstem Planning Unit of the Lower St. Johns Basin. The river was verified as impaired for nutrients during the Cycle 1 assessment and was included on the Verified List of impaired waters for the Lower St. Johns Basin that was adopted by Secretarial Order in May 2004. Based on a total nitrogen (TN)/total phosphorus (TP) median ratio of 8.8 during the Cycle 1 verified period (January 1, 1996–June 30, 2003), nitrogen was identified as the limiting nutrient. Since there were insufficient chlorophyll (chla) data to demonstrate whether the nutrient criterion was being met during the Cycle 2 assessment period, the Arlington River remained on the Verified List for nutrients. This TMDL establishes the allowable loadings to the Arlington River that would restore the waterbody so that it meets its applicable water quality criterion for nutrients.

1.2 Identification of Waterbody

The Arlington River, located in Duval County near downtown Jacksonville (**Figure 1.1**), is a tidally influenced estuarine water that flows into the Lower St. Johns River north of the Hart Bridge. The Arlington River watershed covers 1.58 square miles but receives drainage from a much larger area, including the Pottsburg Creek, Little Pottsburg Creek, Strawberry Creek, and Silversmith Creek sub-basins (**Figure 1.2**).

For assessment purposes, the Florida Department of Environmental Protection (Department) has divided the Lower St. Johns Basin into water assessment polygons with a unique waterbody identification (WBID) number for each watershed or stream reach. This TMDL addresses the Arlington River, WBID 2265A, for nutrients.

The Arlington River is part of the North Mainstem Planning Unit. Planning units are groups of smaller watersheds (WBIDs) that are part of a larger basin unit, in this case the Lower St. Johns Basin. The North Mainstem Planning Unit consists of 49 WBIDs. **Figure 1.3** shows the locations of these WBIDs and the location of the Arlington River watershed in the planning unit.

1.3 Background

This report was developed as part of the Department's watershed management approach for restoring and protecting state waters and addressing TMDL Program requirements. The watershed approach, which is implemented using a cyclical management process that rotates through the state's 52 river basins over a 5-year cycle, provides a framework for implementing the TMDL Program–related requirements of the 1972 federal Clean Water Act and the 1999 Florida Watershed Restoration Act (FWRA) (Chapter 99-223, Laws of Florida).

Figure 1.1. Location of the Arlington River Watershed (WBID 2265A) in the Lower St. Johns Basin



Figure 1.2. Location of the Arlington River Watershed (WBID 2265A) in Duval County and Major Hydrologic Features in the Area





Figure 1.3. WBIDs in the North Mainstem Planning Unit

A TMDL represents the maximum amount of a given pollutant that a waterbody can assimilate and still meet water quality standards, including its applicable water quality criteria and its designated uses. TMDLs are developed for waterbodies that are verified as not meeting their water quality standards. They provide important water quality restoration goals that will guide restoration activities.

A nutrient TMDL, adopted in April 2008 for the mainstem of the Lower St. Johns River, required a 30 to 50 percent reduction in anthropogenic loadings of nitrogen to the marine portion of the Lower St. Johns. A Basin Management Action Plan, or BMAP, adopted in October 2008 outlined a number of activities designed to reduce the amount of TN to the marine portion of the Lower St. Johns River. These activities will depend heavily on the active participation of the St. Johns River Water Management District (SJRWMD), local governments, businesses, and other stakeholders. The Department will work with these organizations and individuals to undertake or continue reductions in the discharge of pollutants and achieve the established TMDLs for impaired waterbodies, including tributaries to the Lower St. Johns such as the Arlington River.

Chapter 2: DESCRIPTION OF WATER QUALITY PROBLEM

2.1 Statutory Requirements and Rulemaking History

Section 303(d) of the federal Clean Water Act requires states to submit to the U.S. Environmental Protection Agency (EPA) lists of surface waters that do not meet applicable water quality standards (impaired waters) and establish a TMDL for each pollutant causing the impairment of listed waters on a schedule. The Department has developed such lists, commonly referred to as 303(d) lists, since 1992. The list of impaired waters in each basin, referred to as the Verified List, is also required by the FWRA (Subsection 403.067[4], Florida Statutes [F.S.]); the state's 303(d) list is amended annually to include basin updates.

Florida's 1998 303(d) list included 55 waterbodies in the Lower St. Johns Basin. However, the FWRA (Section 403.067, F.S.) stated that all previous Florida 303(d) lists were for planning purposes only and directed the Department to develop, and adopt by rule, a new science-based methodology to identify impaired waters. After a long rulemaking process, the Environmental Regulation Commission adopted the new methodology as Rule 62-303, Florida Administrative Code (F.A.C.) (Identification of Impaired Surface Waters Rule, or IWR), in April 2001; the rule was modified in 2006 and 2007.

2.2 Information on Verified Impairment

The Department used the IWR to assess water quality impairments in the Arlington River watershed and has verified that this waterbody segment is impaired for nutrients, based on data in the Department's IWR database. **Tables 2.1** and **2.2** summarize the chl*a* data for the Cycle 1 verified period, which for Group 2 waters was January 1, 1996, through June 30, 2003.

The IWR listing threshold for nutrients in estuaries is based on an annual average chla concentration. Annual average chla in 2002 exceeded the threshold of 11 micrograms per liter (μ g/L). Note that the original calculation of 83.8 μ g/L included a nondetect value that had the incorrect reporting units. The error was subsequently corrected, and the corrected annual average was 13.2 μ g/L. During the Cycle 2 assessment period, an annual average of 6.5 μ g/L was calculated for 2007.

Possible relationships between chla and other water quality parameters are further assessed in Chapter 5, using the complete historical dataset.

Table 2.1. Summary of Corrected Chlorophyll *a* (CHLAC) Monitoring Data for the Arlington River (WBID 2265A) During the Cycle 1 Verified Period (January 1, 1996–June 30, 2003)

- = Empty cell		
Waterbody (WBID)	Parameter	CHLAC (µg/L)
Arlington River (2265A)	Total number of samples	9
Arlington River (2265A)	IWR-annual average threshold for the Verified List	11
Arlington River (2265A)	Number of observed exceedances	1
Arlington River (2265A)	Number of observed nonexceedances	0
Arlington River (2265A)	Number of seasons during which samples were collected	4
Arlington River (2265A)	Annual average resulting in listing (μ g/L)	83.8 (corrected to 13.2)
Arlington River (2265A)	Lowest individual observation (μ g/L)	0
Arlington River (2265A)	Highest individual observation (μ g/L)	850 (corrected to 44)
Arlington River (2265A)	Median TN/TP ratio for 9 observations	9.3
Arlington River (2265A)	Possible causative pollutant by IWR	TN
	FINAL ASSESSMENT:	Impaired

Table 2.2. Summary of Annual Average CHLAC by Year for the Cycle 1Verified Period (January 1, 1991–June 30, 2003)

CHLAC is in µg/L.

Year	Number of Samples	Minimum	Maximum	Annual Mean	Number of Exceedances	Mean Precipitation
2002	8	0.85	850 (corrected to 44)	83.8 (corrected to 13.2)	1	54.72

Chapter 3. DESCRIPTION OF APPLICABLE WATER QUALITY STANDARDS AND TARGETS

3.1 Classification of the Waterbody and Criteria Applicable to the TMDL

Florida's surface waters are protected for five designated use classifications, as follows:

Class I	Potable water supplies
Class II	Shellfish propagation or harvesting
Class III	Recreation, propagation, and maintenance of a healthy, well- balanced population of fish and wildlife
Class IV	Agricultural water supplies
Class V	Navigation, utility, and industrial use (there are no state waters currently in this class)

The Arlington River (WBID 2265A) is a Class III marine waterbody, with a designated use of recreation, propagation, and the maintenance of a healthy, well-balanced population of fish and wildlife. The Class III water quality criterion applicable to the impairment addressed by this TMDL is for nutrients.

3.2 Applicable Water Quality Standards and Numeric Water Quality Target

The nutrient criterion in Rule 62-302, F.A.C., is expressed as a narrative:

Nutrients:

In no case shall nutrient concentrations of a body of water be altered so as to cause an imbalance in natural populations of aquatic flora or fauna [Note: For Class III waters in the Everglades Protection Area, this criterion has been numerically interpreted for phosphorus in Section 62-302.540, F.A.C.].

To assess whether this narrative criterion was being exceeded, the IWR provides thresholds for nutrient impairment in estuaries based on annual average chl*a* levels. The following language is found in Rule 62-303, F.A.C.:

62-303.353 Nutrients in Estuaries and Open Coastal Waters.

Estuaries, estuary segments, or open coastal waters shall be included on the planning list for nutrients if their annual mean chlorophyll a for any year is greater than 11 ug/l or if data indicate annual mean chlorophyll a values have increased by more than 50% over historical values for at least two consecutive years.

62-303.450 Interpretation of Narrative Nutrient Criteria.

(1) A water shall be placed on the verified list for impairment due to nutrients if there are sufficient data from the last five years preceding the planning list

assessment, combined with historical data (if needed to establish historical chlorophyll a levels or historical TSIs), to meet the data sufficiency requirements of subsection 62-303.350(2), FA.C. If there are insufficient data, additional data shall be collected as needed to meet the requirements. Once these additional data are collected, the Department shall determine if there is sufficient information to develop a site-specific threshold that better reflects conditions beyond which an imbalance in flora or fauna occurs in the water segment. If there is sufficient information, the Department shall reevaluate the data using the site-specific thresholds. If there is insufficient information, the Department shall re-evaluate the data using the thresholds provided in Rules 62-303.351-.353, F.A.C., for streams, lakes, and estuaries, respectively. In any case, the Department shall limit its analysis to the use of data collected during the five years preceding the planning list assessment and the additional data collected in the second phase. If alternative thresholds are used for the analysis, the Department shall provide the thresholds for the record and document how the alternative threshold better represents conditions beyond which an imbalance in flora or fauna is expected to occur.

The annual average chla concentration in 2002 exceeded the IWR estuarine threshold of 11 μ g/L in 2002, and, based on the TN/TP ratio, nitrogen was identified as the limiting nutrient. The only additional year during the Cycle 2 verified period with sufficient data to calculate annual average chla was 2007, and chla was below the threshold for impairment.

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Chapter 4: ASSESSMENT OF SOURCES

4.1 Types of Sources

An important part of the TMDL analysis is the identification of pollutant source categories, source subcategories, or individual sources of pollutants in the watershed and the amount of pollutant loading contributed by each of these sources. Sources are broadly classified as either "point sources" or "nonpoint sources." Historically, the term "point sources" has meant discharges to surface waters that typically have a continuous flow via a discernable, confined, and discrete conveyance, such as a pipe. Domestic and industrial wastewater treatment facilities (WWTFs) are examples of traditional point sources. In contrast, the term "nonpoint sources" was used to describe intermittent, rainfall-driven, diffuse sources of pollution associated with everyday human activities, including runoff from urban land uses, agriculture, silviculture, and mining; discharges from failing septic systems; and atmospheric deposition.

However, the 1987 amendments to the Clean Water Act redefined certain nonpoint sources of pollution as point sources subject to regulation under the EPA's National Pollutant Discharge Elimination System (NPDES) Program. These nonpoint sources included certain urban stormwater discharges, including those from local government master drainage systems, construction sites over five acres, and a wide variety of industries (see **Appendix A** for background information on the federal and state stormwater programs).

To be consistent with Clean Water Act definitions, the term "point source" will be used to describe traditional point sources (such as domestic and industrial wastewater discharges) **AND** stormwater systems requiring an NPDES stormwater permit when allocating pollutant load reductions required by a TMDL (see **Section 6.1**). However, the methodologies used to estimate nonpoint source loads do not distinguish between NPDES stormwater discharges and non-NPDES stormwater discharges, and as such, this source assessment section does not make any distinction between the two types of stormwater.

4.2 Potential Sources of Nutrients in the Arlington River Watershed

4.2.1 Point Sources

There are no NPDES wastewater facilities located in the Arlington River WBID.

Municipal Separate Storm Sewer System Permittees

The city of Jacksonville and Florida Department of Transportation (FDOT) District 2 are copermittees for a Phase I NPDES municipal separate storm sewer system (MS4) permit (FLS000012) that includes all of the Arlington River watershed.

4.2.2 Land Uses and Nonpoint Sources

Nutrient loadings to the Arlington River are generated from nonpoint sources in the watershed. These potential sources include loadings from surface runoff, ground water inflow, and septic tanks.

Land Uses

The spatial distribution and acreage of different land use categories were identified using the SJRWMD's year 2004 land use coverage (scale 1:51,000) contained in the Department's geographic information system (GIS) library. Land use categories in the watershed were aggregated using Level 2 land use codes and tabulated in **Table 4.1**. **Figure 4.1** shows the acreage of the principal Level 1 land uses in the watershed.

As shown in **Table 4.1**, the total area of the Arlington River watershed is about 1,011 acres. The dominant land use is urban and built-up, which accounts for about 65.8 percent of the total watershed area. Within this category, low- and medium-density residential along with commercial land use dominate and occupy 56.4 percent of the total watershed area. Of the 981 acres of urban lands, residential land use occupies about 507 acres, or about 6.9 percent of the total watershed area. Natural land uses, including water/wetlands, and upland forest, occupy about 285.7 acres, accounting for about 28.2 percent of the total watershed area.

Table 4.1. Classification of Land Use Categories in the Arlington RiverWatershed (WBID 2265A) in 2004

Level 2 Land Use Code	Attribute	Acres	% of Total
1100	Residential, low density – less than 2 dwelling units/acre	140.25	13.86%
1200	Residential, medium density – 2-5 dwelling units/acre	283.8	28.05%
1300	Residential, high density – 6 or more dwelling units/acre	46.71	4.62%
1400	Commercial and services	147.08	14.54%
1700	Institutional	45.08	4.46%
1800	Recreational	2.46	0.24%
4100	Upland coniferous forests	9.81	0.97%
4300	Upland hardwood forests cont.	18.62	1.84%
5100	Streams and waterways	219.9	21.74%
5300	Reservoirs – pits, retention ponds, dams	2.87	0.28%
6400	Vegetated nonforested wetlands	34.47	3.41%
8100	Transportation	55.62	5.50%
8300	Utilities	4.97	0.49%
-	SUM:	1,011.64	100.00%

Figure 4.1. Principal Land Uses in the Arlington River Watershed (WBID 2265A) in 2004



Soil Characteristics

The Soil Survey Geographic Database (SSURGO) in the Department's GIS database from the SJRWMD was accessed to provide coverage of hydrologic soil groups in the Arlington River watershed (**Figure 4.2**). **Table 4.2** briefly describes the major hydrology soil classes. Soil groups B/D (40.6 percent) and C/D (29.6 percent) are the most common in the watershed, with Type D (28.8 percent) found in the lower portion of the watershed and along its eastern boundaries.

Table 4.2. Description of Hydrologic Soil Classes from the SSURGO Database

Hydrology Class	Description	
А	High infiltration rates. Soils are deep, well-drained to excessively drained sands and gravels.	
A/D	Drained/undrained hydrology class of soils that can be drained and are classified.	
В	Moderate infiltration rates. Deep and moderately deep, moderately well- and well-drained soils that have moderately coarse textures.	
B/D	Drained/undrained hydrology class of soils that have moderately coarse textures.	
С	Slow infiltration rates. Soils with layers impeding downward movement of water, or soils that have moderately fine or fine textures.	
C/D	Drained/undrained hydrology class of soils that can be drained and classified.	
D	Very slow infiltration rates. Soils are clayey, have a high water table, or are shallow to an impervious layer.	

Population

Population and housing unit information from the 2000 census at the block level were obtained from the U.S Census Bureau. GIS was used to estimate the fraction of each block in the Arlington River watershed and then applied to the block information to estimate the population and number of housing units. Based on **Table 4.3**, the population in the watershed is estimated at 3,657 people living in 1,604 households. Based on 2008 Florida Department of Health (FDOH) onsite sewage GIS coverage (available: http://www.doh.state.fl.us/environment/ programs/EhGis/EhGisDownload.htm), about 300 housing units (*N*) were identified as being on septic tanks in the Arlington River watershed (**Figure 4.3**).

Figure 4.2. Distribution of Hydrologic Soil Groups in the Arlington River Watershed (WBID 2265A)



Figure 4.3. Distribution of Onsite Sewage Disposal Systems (Septic Tanks) in the Arlington River Watershed (WBID 2265A)



Table 4.3. Estimated Average Household Size in the Arlington RiverWatershed (WBID 2265A)

-	=	Em	ptv	cell
	_		P'.	00.

Data from U.S. Census Bureau Website, 2000, based Duval County blocks present in the Arlington River watershed.

Tract	Block Group	Population	Housing Units
153	1	15	6
153	2	105	69
154	1	3	3
154	2	430	181
154	3	91	55
155	3	814	337
156	1	1,017	408
156	2	1,059	477
156	3	107	60
157	1	8	5
157	2	9	4
	Total:	3,657	1,604
-	-	AVERAGE HOUSEHOLD SIZE:	2.28

Septic Tanks

Approximately 78 percent of Duval County residences are connected to a wastewater treatment plant, while the rest are using septic tanks (PBS&J, 2007; FDOH Website, 2008). Based on the 2000 census estimates of 1,604 households in the Arlington River WBID, this corresponds to about 352 residences on septic tanks. This estimate is very similar to the 300 residences identified from the 2008 FDOH coverage. Assuming that 300 residences in the Arlington River watershed are using septic tanks, and using an estimate of 70 gallons/day/person (EPA, 1999), and drainfield TN and TP concentrations of 36 and 15 milligrams per liter (mg/L), respectively, potential annual ground water loads of TN and TP were calculated. This is a screening level calculation, and soil types, the age of the system, vegetation, proximity to a receiving water, and other factors will influence the degree of attenuation of this load (**Table 4.4**).

Table 4.4. Estimated Nitrogen and Phosphorus Annual Loading from SepticTanks in the Arlington River Watershed (WBID 2265A)

¹ U.S. Census Bureau; see **Table 4.3** for more information on this estimate.

² EPA, 1999

Estimated Number of Households on Septic	Estimated Number of People per Household ¹	Gallons/ Person/ Day ²	TN in Drainfield (mg/L)	TP in Drainfield (mg/L)	Estimated Annual TN Load (Ibs/yr)	Estimated Annual TP Load (Ibs/yr)
300	2.28	70	36	15	5,251	2,188

4.3.1 Summary of Nutrient Loadings to the Arlington River from Various Sources

Screening level estimates of annual nitrogen and phosphorus loadings to the watershed were developed based on 2004 land use and hydrologic soil groups. GIS shapefiles of land use and hydrologic soil groups were used to determine the acreage associated with various Level 2 land uses and soils. Estimates for annual runoff coefficients and event mean concentrations (EMCs) were based on Harper and Baker (2007) and Gao (2006). A screening level estimate of annual runoff was calculated by multiplying the long-term annual average rainfall of 52.44 inches (Jacksonville International Airport [JIA], 1955–2007) by the respective runoff coefficient and area. Estimates of annual nitrogen and phosphorus loading were obtained by multiplying the annual runoff by the corresponding EMC. A more detailed loading analysis could be performed based on the development of site-specific runoff coefficients, EMCs, and knowledge of best management practices (BMPs) that have been implemented in the watershed.

Agriculture

At the Level 3 land use category, no agricultural areas or rangeland were identified in the Arlington River watershed.

Urban Areas

There are 665 acres in the Level 1 category of urban and built-up in the watershed and 60 acres in transportation, communication, and utilities. Low- and medium-density residential represent approximately 21 and 43 percent, respectively, of the total acreage in the urban and built-up category. **Table 4.5** summarizes the screening level estimates for nitrogen and phosphorus loads from urban and built-up land uses in the watershed.

Forest/Wetland/Water/Open Lands

Table 4.6 summarizes the estimates for nitrogen and phosphorus loadings from the Level 2 land use classifications for forest, wetland, and water. Wetlands and upland forests represent 3.4 and 2.8 percent, respectively, of the acreage in the watershed.

Table 4.5. Estimated Urban and Built-up Annual Nitrogen and PhosphorusLoading in the Arlington River Watershed (WBID 2265A)

				Creas	Estimated TN	Estimated TD
Land Lise	Soil		Annual Runoff	Bunoff		
Classification	Group	Acres	Coefficient	(acre-feet)	(lbs)	(lbs)
Residential, low	0.000				()	()
density - less than 2	D	0.3	0.226	0.30	1.30	0.15
dwelling units/acre						
-	U	131.83	0.435	250.60	1,097.84	130.24
-	С	2.4	0.166	1.74	7.63	0.90
-	W	5.73	0.435	10.89	47.72	5.66
Residential, medium density – 2-5 dwelling units/acre	D	0.24	0.252	0.26	1.49	0.24
-	U	281.02	0.435	534.20	3,008.89	475.32
-	W	2.52	0.435	4.79	26.98	4.26
Residential, high density – 6 or more dwelling units/acre	U	46.12	0.435	87.67	553.45	62.50
-	W	0.6	0.435	1.14	7.20	0.81
Commercial and services	U	146.79	0.435	279.04	1,359.09	198.93
-	W	0.29	0.435	0.55	2.69	0.39
Institutional	U	44.89	0.435	85.33	278.63	60.37
-	D	0.05	0.35	0.08	0.25	0.05
-	W	0.14	0.435	0.27	0.87	0.19
Recreational	U	2.46	0.435	4.68	14.63	0.70
Transportation	U	55.48	0.435	105.46	470.63	63.13
-	W	0.17	0.435	0.32	1.44	0.19
-	U	4.97	0.435	9.45	42.16	5.66
-	SUM:	726	-	1,376.78	6,922.89	1,009.71

- = Empty cell/no data

Table 4.6. Estimated Forest/Wetland/Water/Open Lands Annual Nitrogenand Phosphorus Loading in the Arlington River Watershed (WBID2265A)

Land Use Classification	Soil Group	Acres	Annual Runoff Coefficient	Gross Runoff (acre-feet)	Estimated TN Load (Ibs)	Estimated TP Load (Ibs)
Upland coniferous forests	W	0.3	0.435	0.57	1.78	1.78
-	U	8.78	0.435	16.69	52.23	52.23
-	С	0.73	0.166	0.53	1.66	1.66
Upland hardwood forests	U	16.29	0.435	30.97	96.90	96.90
-	W	1.17	0.435	2.22	6.96	6.96
-	С	1.15	0.166	0.83	2.61	2.61
Streams and waterways	С	0.79	0.435	1.50	5.11	0.45
-	U	9.44	0.435	17.94	61.04	5.37
-	D	3.57	0.435	6.79	23.08	2.03
-	W	206.1	0.435	391.79	1,332.56	117.27
Reservoirs – pits, retention ponds, dams	W	2.04	0.435	3.88	13.19	1.16
-	U	0.33	0.435	0.63	2.13	0.19
-	С	0.5	0.435	0.95	3.23	0.28
Vegetated nonforested wetlands	D	30.64	0.435	58.25	253.58	9.51
-	U	1.37	0.435	2.60	11.34	0.43
-	W	2.45	0.435	4.66	20.28	0.76
-	SUM:	285.65	-	540.80	1,887.67	299.58

- = Empty cell/no data

Upstream Areas Draining to the Arlington River Watershed

The Red Bay Branch, Silversmith Creek, Strawberry Creek, Pottsburg Creek, and Little Pottsburg Creek sub-basins contribute to the water quality observed in the Arlington River (**Figure 4.4**). The same procedure used for the Arlington River WBID was used to estimate annual TN and TP loading from these contributing watersheds (**Table 4.7**).

Figure 4.4. Upstream Areas Draining to the Arlington River Watershed (WBID 2265A)



Table 4.7. Estimated Annual Nitrogen and Phosphorus Loading to the
Arlington River Watershed (WBID 2265A) from Upstream
Drainage Areas

Land Use Category	Acres	Gross Runoff (acre-feet)	Estimated TN Load (Ibs)	Estimated TP Load (Ibs)
Urban	11,036.55	18,199.62	95,337.89	15,680.86
Agriculture	205.55	144.39	532.79	40.17
Forest/Wetland/ Water	2,500.1	4,064.65	16,265.91	792.61
TOTAL:	13,742.2	22,408.67	112,136.60	16,513.65

Table 4.8 summarizes the estimates from various land uses in the watershed. It is important to note that this is not a complete list and represents estimates of potential loadings. In addition, proximity to the waterbody, site-specific soil characteristics, and rainfall frequency and magnitude are just a few of the factors that could influence and determine the actual loadings from these sources that reach Arlington River. The types of BMPs, both structural and nonstructural, implemented for specific land uses in the watershed could reduce the actual nutrient loads delivered to the Arlington River. Finally, the age and condition of the septic systems and drainage characteristics in the watershed compared with the county overall could affect assumptions about the assimilation and/or retention of nutrients.

Table 4.8. Summary of Estimated Potential Annual Nitrogen andPhosphorus Loading from Various Sources in the Arlington RiverWatershed (WBID 2265A)

Source	TN (lbs/yr)	TP (lbs/yr)
Septic Tanks ¹	5,251	2,188
Urban and Built-up	6,922.89	1,009.71
Forest/Wetland/Water/ Open Lands	1,887.67	299.58
Upstream Drainage Area	112,136.60	16,513.65

¹ Potential contribution to ground water

The screening model estimated an annual surface runoff of 24,326.2 acre-feet, or 19.8 inches per year, based on the combined Arlington River watershed and contributing watershed area. Dividing the estimated TN load by the surface runoff volume yielded an average TN concentration of 1.83 mg/L. The average and median TN concentrations from the available data were 1.15 and 1.14 mg/L, respectively. Dividing the estimated TP load by the surface runoff volume yielded an average and median TP concentrations from the available data were 0.176 and 0.140 mg/L, respectively. Flow and nutrient contributions from ground water inputs to the Arlington River were not included in this screening level calculation and would likely influence instream concentrations. Tidal exchange with the St. Johns River was also not considered in the calculation.

Camp Dresser & McKee, Inc. is currently working with the city of Jacksonville on an update to the Master Stormwater Management Plan, using the Watershed Management Model (WMM) to develop nutrient loads for sub-basins. The Arlington River watershed is one of the watersheds in which the WMM is being applied.

Chapter 5: DETERMINATION OF ASSIMILATIVE CAPACITY

5.1 Determination of Loading Capacity

5.1.1 Data Used in the Determination of the TMDL

Twelve sampling stations on the Arlington River have historical observations (**Figure 5.1**). Six of those stations have CHLAC measurements. **Table 5.1** contains summary information on each of the stations (N represents the number of CHLAC observations). **Table 5.2** provides a statistical summary of CHLAC observations at each station, and **Appendix B** contains CHLAC, temperature (TEMP), TN, and TP observations from sampling sites in WBID 2265A from 1992 to 2008. **Figure 5.2** displays the historical CHLAC observations over time. The simple linear regression of CHLAC versus sampling date was significant at an α level of 0.05. **Appendix E** contains plots of CHLAC by season, station, and year.

Figures 5.3 and **5.4** present historical TN and TP observations, respectively. Linear regressions of each parameter versus sampling date indicate that regressions for TN and TP were significant at an α level of 0.05. **Appendix E** contains additional plots by season, station, and year. **Tables 5.3** and **5.4** provide statistical summaries of TN and TP, respectively, by station. **Table 5.5** presents a statistical summary of major water quality parameters from the available data.

Station	STORET ID	Station Owner	Years With Data	N
Arlington River at the St. Johns River	11NPSWRDTIMU_DE R_SJR-11	Department	1982–83	0
Arlington R Cesery Blvd	21FLA 20030073	Department	1968–2008	17
Arlington River Near Mouth Below Pottsburg Ck	21FLJXWQARLRM	City of Jacksonville	1994–95	0
Arlington R. Near Confl. With L. Pottsburg Cr.	21FLJXWQJAXSJR20 A	City of Jacksonville	1999–2001	0
Arlington River at Cesery Blvd	21FLJXWQRR0005	City of Jacksonville	1983–91	0
Arlington River at University Blvd	21FLSJWMLSJ090	SJRWMD	1992–93	4
Arlington River Near Mouth Below Pottsburg Ck	21FLSJWMARLRM	SJRWMD	1993–94	2
Arlington R Nr Mouth Bel Pottsburg Ck Stormflow	21FLSJWMARLRM.S	SJRWMD	1993–94	0
Arlington River Bridge	21FLVOL ARL010	Volunteer	1996	0
Arlington R Nr Conf w/ Little Pottsburg	21FLA 20030811	Department	2007–08	5
Arlington R 500M SW University Blvd	21FLA 20030812	Department	2007	5
Arlington R Btwn Conf W Strawberry Cr & Silversmith Cr	21FLA 20030813	Department	2007	5

Table 5.1. Sampling Station Summary for the Arlington River (WBID 2265A)

Table 5.2. Statistical Summary of Historical CHLAC Data for the Arlington River (WBID 2265A)

CHLAC data are μg/L.					
Station	N	Minimum	Maximum	Mean	Median
Arlington R Cesery Blvd	17	1.0	44.0	9.2	1.0
Arlington River at University Blvd	4	1.9	71.6	28.2	19.6
Arlington River Near Mouth Below Pottsburg Ck	2	4.8	19.2	12.0	12.0
Arlington R Nr Conf w/ Little Pottsburg	5	1.0	16.0	6.2	5.1
Arlington R 500M SW University Blvd	5	1.0	12.0	6.5	8.7
Arlington R Btwn Conf W Strawberry Cr & Silversmith Cr	5	1.0	16.0	10.8	12.0

Table 5.3. Statistical Summary of Historical TN Data for the Arlington River (WBID 2265A)

TN data are mg/L.

Station	Ν	Minimum	Maximum	Mean	Median
Arlington R Cesery Blvd	54	0.17	3.09	1.33	1.31
Arlington River at Cesery Blvd	14	0.27	1.62	1.04	1.07
Arlington River at University Blvd	4	0.62	1.19	1.00	1.10
Arlington River Near Mouth Below Pottsburg Ck	8	0.18	1.44	0.87	0.96
Arlington River Near Mouth Below Pottsburg Ck	4	1.08	1.71	1.38	1.36
Arlington River Near Mouth Below Pottsburg Ck (Jax)	6	0.60	0.97	0.78	0.77
Arlington R. Near Confl. With L. Pottsburg Cr.	32	0.31	2.53	1.03	0.94
Arlington R Nr Conf w/ Little Pottsburg	5	0.63	1.74	1.20	1.25
Arlington R 500M SW University Blvd	5	0.66	1.57	1.14	1.17
Arlington R Btwn Conf W Strawberry Cr & Silversmith Cr	5	0.69	1.70	1.19	1.19

Table 5.4. Statistical Summary of Historical TP Data for the Arlington River(WBID 2265A)

TP data are mg/L.					
Station	Ν	Minimum	Maximum	Mean	Median
Arlington R Cesery Blvd	50	0.027	0.850	0.210	0.135
Arlington River at Cesery Blvd	34	0.030	0.351	0.165	0.152
Arlington River at University Blvd	4	0.130	0.190	0.150	0.140
Arlington River Near Mouth Below Pottsburg Ck	9	0.110	0.180	0.131	0.120
Arlington River Near Mouth Below Pottsburg Ck	5	0.147	0.327	0.209	0.187
Arlington River Near Mouth Below Pottsburg Ck (Jax)	6	0.091	0.248	0.137	0.111
Arlington R Nr Conf w/ Little Pottsburg	5	0.082	0.210	0.146	0.150
Arlington R 500M SW University Blvd	5	0.091	0.210	0.136	0.140
Arlington R Btwn Conf W Strawberry Cr & Silversmith Cr	5	0.110	0.200	0.148	0.150

PARM	N	MIN	25%	MEDIAN	MEAN	75%	MAX
BOD (mg/L)	137	0.0	1.1	2.0	1.8	2.0	7.8
CHLAC (ug/L)	38	1.0	1.0	7.4	10.8	15.0	71.6
CHLORIDE (mg/L)	176	4	1000	2687	3581	5484	17994
COLOR (PCU)	110	30	50	60	83	100	225
COND (uS/cm)	164	14	4527	9630	11514	17850	45350
DO (mg/L)	256	1.50	5.86	6.90	7.01	8.20	19.50
DOSAT (%)	240	31.77	69.37	83.17	82.93	93.52	229.41
NH4 (mg/L)	169	0.00	0.01	0.03	0.08	0.08	1.13
NO3O2 (mg/L)	124	0.00	0.04	0.16	0.17	0.28	0.53
PH (su)	248	5.10	7.11	7.48	7.40	7.70	8.83
SO4 (mg/L)	38	24.00	230.00	510.00	589.76	916.00	1 400.00
TEMP (C)	249	7.00	19.15	24.00	23.34	27.85	33.50
TKN (mg/L)	102	0.13	0.63	0.87	0.88	1.07	2.37
TN (mg/L)	137	0.17	0.75	1.14	1.15	1.44	3.09
TOC (mg/L)	54	5.00	10.00	13.00	15.46	18.00	60.00
TORTHO (MG/L)	59	0.040	0.060	0.089	0.122	0.104	1.030
TP (mg/L)	123	0.027	0.110	0.140	0.178	0.204	0.850
TSS (mg/L)	152	4	15	19	23	29	161
TURB (NTU)	119	1	4	6	8	10	70

Table 5.5. Summary Statistics for Major Water Quality ParametersMeasured in the Arlington River (WBID 2265A)

Figure 5.1. Historical Sampling Sites in the Arlington River (WBID 2265A)







Figure 5.3. Historical TN Observations for the Arlington River (WBID 2265A)





Figure 5.4. Historical TP Observations for the Arlington River (WBID 2265A)

Available CHLAC, TN, and TP measurements were also summarized by year (**Tables 5.6, 5.7**, and **5.8**). A nonparametric test (Kruskal-Wallis) was applied to the CHLAC, TN, and TP datasets to determine whether there were significant difference among seasons (**Appendix C**). At an α level of 0.05, differences were significant among seasons for TN. A similar test for differences among months was not significant at an α level of 0.05 (**Appendix D**). Time series plots of TN (**Figure 5.3**) and TP (**Figure 5.4**) suggested a decline in concentrations, particularly when compared with levels observed during the mid-1970s.

Table 5.6. Statistical Summary of Historical CHLAC Data by Year for the Arlington River (WBID 2265A)

Year	N	Minimum	Maximum	Mean	Median
1984	1	37.3	37.3	37.3	37.3
1985	1	7.0	7.0	7.0	7.0
1992	3	13.9	71.6	37.0	25.4
1993	3	1.9	19.2	8.6	4.8
2002	10	1.0	44.0	7.1	1.0
2007	20	1.0	16.0	8.0	9.3

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CHLAC concentrations are µg/L.
Table 5.7. Statistical Summary of Historical TN Data by Year for the
Arlington River (WBID 2265A)

Year	N	Minimum	Maximum	Mean	Median
1973	1	1.05	1.05	1.05	1.05
1974	8	1.38	2.40	1.74	1.69
1975	8	0.59	3.08	1.48	1.43
1976	7	0.17	1.67	1.19	1.28
1977	6	0.71	2.57	1.41	1.23
1978	5	0.41	3.09	1.29	0.67
1979	1	1.62	1.62	1.62	1.62
1983	1	0.88	0.88	0.88	0.88
1984	3	0.39	1.15	0.77	0.75
1985	1	0.32	0.32	0.32	0.32
1988	1	1.15	1.15	1.15	1.15
1989	5	0.27	1.27	0.97	1.10
1990	4	0.51	1.62	1.16	1.25
1991	2	0.95	1.32	1.14	1.14
1992	3	0.62	1.18	0.94	1.02
1993	7	0.41	1.46	1.00	1.01
1994	8	0.18	1.71	1.02	1.03
1995	4	0.67	0.92	0.78	0.77
1999	10	0.51	1.83	0.83	0.68
2000	11	0.31	2.53	1.20	1.18
2001	11	0.62	1.84	1.03	0.95
2002	9	0.82	2.02	1.27	1.22
2007	20	0.63	1.74	1.17	1.20
2008	1	1.09	1.09	1.09	1.09

TN concentrations are mg/L

Table 5.8. Statistical Summary of Historical TP Data by Year for theArlington River (WBID 2265A)

Year	N	Minimum	Maximum	Mean	Median
1974	8	0.133	0.636	0.374	0.357
1975	5	0.059	0.850	0.474	0.540
1976	9	0.027	0.357	0.113	0.062
1977	5	0.127	0.238	0.165	0.164
1978	4	0.041	0.081	0.055	0.049
1979	1	0.220	0.220	0.220	0.220
1983	1	0.316	0.316	0.316	0.316
1984	7	0.043	0.419	0.175	0.131
1985	6	0.085	0.309	0.188	0.162
1986	5	0.094	0.209	0.147	0.128
1987	4	0.141	0.351	0.205	0.165
1988	3	0.096	0.201	0.143	0.132
1989	5	0.068	0.330	0.194	0.182
1990	4	0.110	0.210	0.165	0.170
1991	2	0.030	0.090	0.060	0.060
1992	3	0.130	0.190	0.157	0.150
1993	8	0.110	0.327	0.152	0.130
1994	9	0.091	0.205	0.150	0.147
1995	4	0.102	0.248	0.153	0.131
2002	9	0.099	0.270	0.141	0.130
2007	20	0.082	0.210	0.143	0.145
2008	1	0.110	0.110	0.110	0.110

TP concentrations are mg/L

5.1.2 TMDL Development Process

A Spearman correlation matrix was used to assess the potential relationships between CHLAC and other water quality parameters (**Appendix G**). At an alpha (α) level of 0.05, correlations between CHLAC and the water quality parameters BOD5, conductance, pH, and salinity were significant. The simple linear regression equations suggested that increased conductance, salinity, or pH resulted in lower CHLAC concentrations (**Appendix H**).

Rainfall records for JIA (**Appendix F** illustrates rainfall from 1955 to 2008) were used to determine the rainfall amounts associated with individual sampling dates. Rainfall recorded on the day of sampling (1D), the cumulative total for the day of and the previous 2 days (3D), the cumulative total for the day of and the previous 6 days (7D), and the cumulative total for the day of sampling and 13 days prior (14D) were all paired with the respective water quality observation based on date. Simple linear regressions of CHLAC versus precip, precip3day, precip7day, and precip14day were all significant at an α level of 0.05. Among the four

variables, the precip3day explained the most variance in CHLAC (33 percent). The expression suggested a positive relationship between the cumulative rainfall total and CHLAC. This could represent a lag in the delivery of algal biomass from the upstream drainage area and/or a lag in the response of algae to an increase in available nutrients.

The general linear model (GLM) was used to evaluate possible relationships between nutrients that included the interaction with TEMP. None of the expressions involving TEMP, TN, or TP separately or in combination was significant at an α level of 0.05.

The Arlington River is tidally influenced, and there is a 3- to 4-foot tidal variation in the Jacksonville area. Previous dye studies conducted for the Buckman WWTF discharge to the St. Johns River north of the Hart Bridge demonstrated the exchange between the St. Johns and the Arlington River. Historical CHLAC, TN, and TP measurements from the St. Johns River (WBID 2213D) near the Arlington River were compared with measurements from the Arlington River (**Figures 5.5** through **5.7**, respectively). Fluctuations in CHLAC, TN, and TP concentrations in the St. Johns River are reflected in the Arlington River.

Figure 5.5. Comparison of Historical CHLAC Measurements from the St. Johns River (WBID 2213D) and the Arlington River (WBID 2265A)



Figure 5.6. Comparison of Historical TN Measurements from the St. Johns River (WBID 2213D) and the Arlington River (WBID 2265A)



Figure 5.7. Comparison of Historical TP Measurements from the St. Johns River (WBID 2213D) and the Arlington River (WBID 2265A)



Plots of CHLAC, TN, and TP versus sample date in the Arlington River indicated a pattern of decreasing concentrations. A cumulative frequency plot of historical CHLAC concentrations (**Appendix E**) demonstrated that 30 percent of the observations were 1 μ g/L or less. Over 60 percent of the observations were below 11 μ g/L. The Arlington River was verified impaired based on exceeding the estuarine annual average chl*a* threshold of 11 μ g/L in one year (13.2 μ g/L in 2002). The next year with sufficient data to calculate an annual average per the IWR was well below the threshold (6.6 μ g/L in 2007).

The adopted nutrient TMDL for the Lower St. Johns River requires a 30 to 50 percent reduction in anthropogenic nitrogen loads to the marine portion of the river. This requirement will influence nitrogen, phosphorus, and CHLAC contributions to the Arlington River from both the contributing watersheds as well as the direct tidal exchange with the St. Johns River. These reductions, in conjunction with the existing trend of declining CHLAC, TN, and TP concentrations in the Arlington River, are expected to restore the river such that designated uses are restored and maintained.

5.1.3 Critical Conditions/Seasonality

A nonparametric test (Kruskal-Wallis) was applied to the CHLAC, TN, and TP datasets to determine whether there were significant differences among months or seasons. At an alpha (α) level of 0.05, there were no significant differences among seasons or months for either CHLAC or TP. The only significant difference found was between TN and season (Appendices C and D). Tables 5.9a, 5.9b, and 5.9c provide seasonal summary statistics for CHLAC, TN, and TP, respectively. Mean and median CHLAC levels were elevated during the summer and fall seasons (Table 5.9a). In comparison, mean and median TN concentrations in the fall and winter seasons were elevated relative to other seasons (Table 5.9b). Mean, median, and maximum TP concentrations were elevated during the summer season relative to other seasons. Table 5.9c provides seasonal summary statistics for TP.

Table 5.9a. Seasonal Summary Statistics for CHLAC for the Arlington River (WBID 2265A)

Season	N	Minimum	5%	25%	Mean	Median	75%	Maximum
Winter	7	1.0	1.0	1.0	8.1	1.0	5.7	44.0
Spring	10	1.0	1.0	1.0	8.5	1.8	10.0	37.3
Summer	9	1.0	1.0	4.1	16.2	10.0	17.6	71.6
Fall	12	1.0	1.0	4.5	10.3	13.0	15.0	16.0

Table 5.9b. Seasonal Summary Statistics for TN for the Arlington River (WBID 2265A)

Season	Ν	Minimum	5%	25%	Mean	Median	75%	Maximum
Winter	32	0.27	0.34	0.73	1.25	1.18	1.56	3.09
Spring	30	0.17	0.18	0.71	0.98	0.90	1.22	2.53
Summer	34	0.31	0.40	0.67	1.08	1.13	1.26	2.40
Fall	41	0.47	0.56	0.95	1.27	1.35	1.56	2.02

Table 5.9c. Seasonal Summary Statistics for TP for the Arlington River (WBID 2265A)

Season	N	Minimum	5%	25%	Mean	Median	75%	Maximum
Winter	29	0.030	0.040	0.101	0.162	0.127	0.173	0.608
Spring	26	0.027	0.053	0.094	0.157	0.115	0.190	0.459
Summer	34	0.029	0.044	0.110	0.213	0.179	0.238	0.850
Fall	34	0.062	0.083	0.130	0.173	0.150	0.180	0.363

TP concentrations are mg/L.

Chapter 6: DETERMINATION OF THE TMDL

6.1 Expression and Allocation of the TMDL

The objective of a TMDL is to provide a basis for allocating acceptable loads among all of the known pollutant sources in a watershed so that appropriate control measures can be implemented and water quality standards achieved. A TMDL is expressed as the sum of all point source loads (wasteload allocations, or WLAs), nonpoint source loads (load allocations, or LAs), and an appropriate margin of safety (MOS), which takes into account any uncertainty concerning the relationship between effluent limitations and water quality:

$\mathsf{TMDL} = \sum \mathsf{WLAs} + \sum \mathsf{LAs} + \mathsf{MOS}$

As discussed earlier, the WLA is broken out into separate subcategories for wastewater discharges and stormwater discharges regulated under the NPDES Program:

$\textbf{TMDL} \cong \sum \textbf{WLAs}_{wastewater} + \sum \textbf{WLAs}_{NPDES \ Stormwater} + \sum \textbf{LAs} + \textbf{MOS}$

It should be noted that the various components of the revised TMDL equation may not sum up to the value of the TMDL because (a) the WLA for NPDES stormwater is typically based on the percent reduction needed for nonpoint sources and is also accounted for within the LA, and (b) TMDL components can be expressed in different terms (for example, the WLA for stormwater is typically expressed as a percent reduction, and the WLA for wastewater is typically expressed as mass per day).

WLAs for stormwater discharges are typically expressed as "percent reduction" because it is very difficult to quantify the loads from MS4s (given the numerous discharge points) and to distinguish loads from MS4s from other nonpoint sources (given the nature of stormwater transport). The permitting of stormwater discharges also differs from the permitting of most wastewater point sources. Because stormwater discharges cannot be centrally collected, monitored, and treated, they are not subject to the same types of effluent limitations as wastewater facilities, and instead are required to meet a performance standard of providing treatment to the "maximum extent practical" through the implementation of BMPs.

This approach is consistent with federal regulations (40 CFR § 130.2[I]), which state that TMDLs can be expressed in terms of mass per time (e.g., pounds per day), toxicity, or **other appropriate measure**. The TMDL for the Arlington River is expressed in terms of a percent reduction in TN and TP, to meet both the DO and nutrient criteria (**Table 6.1**).

Table 6.1. TMDL Components for the Arlington River (WBID 2265A)

- = Empty cell/no data

¹ As the TMDL represents a percent reduction, it also complies with EPA requirements to express the TMDL on a daily basis.

WBID	Parameter	TMDL (mg/L)	WLA for Wastewater (mg/L)	WLA for NPDES Stormwater (% reduction) ¹	LA (% Reduction) ¹	MOS
2265A	TN	-	N/A	30%	30%	Implicit

6.2 Load Allocation

TN reductions of 30 percent are required from nonpoint sources. It should be noted that the load allocation includes loading from stormwater discharges that are not part of the NPDES Stormwater Program.

6.3 Wasteload Allocation

6.3.1 NPDES Wastewater Discharges

There are currently no permitted NPDES discharges in the Arlington River watershed; however, any future discharge permits issued in the watershed will also be required to contain appropriate discharge limitations on nitrogen that will comply with the TMDL.

6.3.2 NPDES Stormwater Discharges

The city of Jacksonville and FDOT District 2 are co-permittees for a Phase I NPDES MS4 permit (FLS000012) that includes all of the Arlington River watershed.

6.4 Margin of Safety

Consistent with the recommendations of the Allocation Technical Advisory Committee (Department, 2001), an implicit MOS was used in the development of this TMDL by considering all available historical CHLAC, TN, and TP data, and the existing mainstem nutrient TMDL that requires a 30 to 50 percent reduction in anthropogenic nitrogen loading.

Chapter 7: NEXT STEPS: IMPLEMENTATION PLAN DEVELOPMENT AND BEYOND

7.1 Basin Management Action Plan

Following the adoption of this TMDL by rule, the Department will determine the best course of action regarding its implementation. Depending on the pollutant(s) causing the waterbody impairment and the significance of the waterbody, the Department will select the best course of action leading to the development of a plan to restore the waterbody. Often this will be accomplished cooperatively with stakeholders by creating a Basin Management Action Plan, referred to as the BMAP. BMAPs are the primary mechanism through which TMDLs are implemented in Florida (see Subsection 403.067[7], F.S.). A single BMAP may provide the conceptual plan for the restoration of one or many impaired waterbodies.

If the Department determines that a BMAP is needed to support the implementation of this TMDL, a BMAP will be developed through a transparent, stakeholder-driven process intended to result in a plan that is cost-effective, technically feasible, and meets the restoration needs of the applicable waterbodies. Once adopted by order of the Department Secretary, BMAPs are enforceable through wastewater and municipal stormwater permits for point sources and through BMP implementation for nonpoint sources. Among other components, BMAPs typically include the following:

- Water quality goals (based directly on the TMDL);
- Refined source identification;
- Load reduction requirements for stakeholders (quantitative detailed allocations, if technically feasible);
- A description of the load reduction activities to be undertaken, including structural projects, nonstructural BMPs, and public education and outreach;
- A description of further research, data collection, or source identification needed in order to achieve the TMDL;
- Timetables for implementation;
- Implementation funding mechanisms;
- An evaluation of future increases in pollutant loading due to population growth;
- Implementation milestones, project tracking, water quality monitoring, and adaptive management procedures; and
- Stakeholder statements of commitment (typically a local government resolution).

BMAPs are updated through annual meetings and may be officially revised every five years. Completed BMAPs in the state have improved communication and cooperation among local stakeholders and state agencies; improved internal communication within local governments; applied high-quality science and local information in managing water resources; clarified the obligations of wastewater point source, MS4, and non-MS4 stakeholders in TMDL implementation; enhanced transparency in the Department's decision making; and built strong relationships between the Department and local stakeholders that have benefited other program areas.

7.2 Other TMDL Implementation Tools

However, in some basins, and for some parameters, particularly those with fecal coliform impairments, the development of a BMAP using the process described above will not be the most efficient way to restore a waterbody, such that it meets its designated uses. This is because fecal coliform impairments result from the cumulative effects of a multitude of potential sources, both natural and anthropogenic. Addressing these problems requires good old-fashioned detective work that is best done by those in the area.

A multitude of assessment tools is available to assist local governments and interested stakeholders in this detective work. The tools range from the simple (such as Walk the WBIDs and GIS mapping) to the complex (such as bacteria source tracking). Department staff will provide technical assistance, guidance, and oversight of local efforts to identify and minimize fecal coliform sources of pollution. Based on work in the Lower St Johns River Tributaries and the Hillsborough Basin, the Department and local stakeholders have developed a logical process and tools to serve as a foundation for this detective work. In the near future, the Department will be releasing these tools to assist local stakeholders with the development of local implementation plans to address fecal coliform impairments. In such cases, the Department will **rely on these local initiatives** as a more cost-effective and simplified approach to identify the actions needed to put in place a road map for restoration activities, while still meeting the requirements of Subsection 403.067(7), F.S.

Earlier in the document, reference was made to the BMAP adopted in October 2008 that outlined implementation activities in the marine portion of the Lower St. Johns River to achieve the nutrient TMDL. Since the Arlington River represents a contributing watershed to the Lower St. Johns River, applicable activities undertaken in the Arlington River watershed as part of the Lower St. Johns River BMAP should be sufficient to address the nutrient impairment in the Arlington River.

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Appendices

Appendix A: Background Information on Federal and State Stormwater Programs

In 1982, Florida became the first state in the country to implement statewide regulations to address the issue of nonpoint source pollution by requiring new development and redevelopment to treat stormwater before it is discharged. The Stormwater Rule, as authorized in Chapter 403, F.S., was established as a technology-based program that relies on the implementation of BMPs that are designed to achieve a specific level of treatment (i.e., performance standards) as set forth in Rule 62-40, F.A.C. In 1994, the Department's stormwater treatment requirements were integrated with the stormwater flood control requirements of the water management districts, along with wetland protection requirements, into the Environmental Resource Permit regulations.

Rule 62-40 also requires the state's water management districts to establish stormwater pollutant load reduction goals (PLRGs) and adopt them as part of a Surface Water Improvement and Management (SWIM) plan, other watershed plan, or rule. Stormwater PLRGs are a major component of the load allocation part of a TMDL. To date, stormwater PLRGs have been established for Tampa Bay, Lake Thonotosassa, the Winter Haven Chain of Lakes, the Everglades, Lake Okeechobee, and Lake Apopka.

In 1987, the U.S. Congress established Section 402(p) as part of the federal Clean Water Act Reauthorization. This section of the law amended the scope of the federal NPDES permitting program to designate certain stormwater discharges as "point sources" of pollution. The EPA promulgated regulations and began implementing the Phase I NPDES Stormwater Program in 1990. These stormwater discharges include certain discharges that are associated with industrial activities designated by specific standard industrial classification (SIC) codes, construction sites disturbing 5 or more acres of land, and the master drainage systems of local governments with a population above 100,000, which are better known as MS4s. However, because the master drainage systems of most local governments in Florida are interconnected, the EPA implemented Phase I of the MS4 permitting program on a countywide basis, which brought in all cities (incorporated areas), Chapter 298 urban water control districts, and the FDOT throughout the 15 counties meeting the population criteria. The Department received authorization to implement the NPDES Stormwater Program in 2000.

An important difference between the federal NPDES and the state's stormwater/environmental resource permitting programs is that the NPDES Program covers both new and existing discharges, while the state's program focus on new discharges only. Additionally, Phase II of the NPDES Program, implemented in 2003, expands the need for these permits to construction sites between 1 and 5 acres, and to local governments with as few as 1,000 people. While these urban stormwater discharges are now technically referred to as "point sources" for the purpose of regulation, they are still diffuse sources of pollution that cannot be easily collected and treated by a central treatment facility, as are other point sources of pollution such as domestic and industrial wastewater discharges. It should be noted that all MS4 permits issued in Florida include a reopener clause that allows permit revisions to implement TMDLs when the implementation plan is formally adopted.

40

0.130

0.027

1.22

-

23

23

Appendix B: CHLAC, TEMP, TN, and TP Observations in the Arlington River (WBID 2265A), 1992–2008

- = Em	pty cell/no data					
	Station	Sample Date	CHLAC (μg/L))	TEMP (°C)	TN (mg/L)	TP (mg/L)
	FLA20030073	11/7/1973	-	21.5	1.05	-
	FLA20030073	1/7/1974	-	20	-	-
	FLA20030073	2/25/1974	-	-	2.05	-
	FLA20030073	3/19/1974	-	19	1.39	0.304
	FLA20030073	4/15/1974	-	-	1.60	-
	FLA20030073	5/13/1974	-	24	1.77	0.459
	FLA20030073	6/12/1974	-	29.5	1.38	-
	FLA20030073	7/15/1974	-	28.3	-	-
	FLA20030073	7/15/1974	-	-	-	0.350
	FLA20030073	8/12/1974	-	-	2.40	0.636
	FLA20030073	9/9/1974	-	-	-	-
	FLA20030073	9/16/1974	-	30	1.55	0.496
	FLA20030073	10/29/1974	-	24	1.79	0.363
	FLA20030073	11/18/1974	-	20	-	0.248
	FLA20030073	12/10/1974	-	18	-	0.133
	FLA20030073	1/6/1975	-	16	-	0.608
	FLA20030073	2/24/1975	-	19.5	-	-
	FLA20030073	3/3/1975	-	13.5	-	-
	FLA20030073	3/10/1975	-	17	3.08	-
	FLA20030073	3/24/1975	-	22	-	-
	FLA20030073	3/31/1975	-	21.5	-	-
	FLA20030073	4/21/1975	-	23.8	0.59	0.313
	FLA20030073	4/28/1975	-	26	-	-
	FLA20030073	5/5/1975	-	24.5	-	-
	FLA20030073	6/2/1975	-	26	0.76	0.059
	FLA20030073	6/30/1975	-	-	-	-
	FLA20030073	7/21/1975	-	-	1.52	-
	FLA20030073	8/4/1975	-	28	1.20	0.850
	FLA20030073	9/8/1975	-	29	-	0.540
	FLA20030073	10/13/1975	-	27	1.84	-
	FLA20030073	11/3/1975	-	-	1.45	-
	FLA20030073	12/15/1975	-	22	1.41	-
	FLA20030073	3/22/1976	-	20.5	1.28	0.357

Florida Department of Environmental Protection

FLA20030073

FLA20030073

-

-

4/19/1976

5/17/1976

Station	Sample Date	CHLAC (ug/L))	TEMP (°C)	TN (mg/L)	TP (mg/L)
FLA20030073	6/21/1976	-	23.5	-	0.059
FLA20030073	7/19/1976	-	24	-	0.136
FLA20030073	8/30/1976	-	28.5	-	0.053
FLA20030073	9/20/1976	-	27.5	-	0.029
FLA20030073	11/1/1976	-	-	1.67	0.062
FLA20030073	12/15/1976	-	17	1.38	0.161
FLA20030073	1/17/1977	-	-	1.56	0.167
FLA20030073	3/14/1977	-	22	2.57	0.127
FLA20030073	5/2/1977	-	27	1.15	0.164
FLA20030073	6/2/1977	-	-	-	-
FLA20030073	6/13/1977	-	20.5	0.71	-
FLA20030073	7/13/1977	-	33.5	1.31	0.238
FLA20030073	12/19/1977	-	18	1.15	0.127
FLA20030073	1/31/1978	-	8	3.09	0.041
FLA20030073	8/8/1978	-	30	1.84	0.050
FLA20030073	9/19/1978	-	29.8	0.41	0.049
FLA20030073	10/18/1978	-	22	0.67	0.081
FLA20030073	11/14/1978	-	22	0.47	-
FLA20030073	12/19/1978	-	15	-	-
FLA20030073	1/11/1979	-	12.5	1.62	0.220
11NPSWRDTIMUDER_SJR-11	4/19/1982	-	23.9	-	-
11NPSWRDTIMUDER_SJR-11	8/9/1982	-	30.1	-	-
FLJXWQRR0005	10/5/1983	-	28.5	-	-
FLJXWQRR0005	12/14/1983	-	16.5	0.88	0.316
FLJXWQRR0005	3/14/1984	-	18.5	0.75	0.122
FLA20030073	4/24/1984	37.295	23	1.15	0.419
FLJXWQRR0005	6/26/1984	-	29	-	0.110
FLJXWQRR0005	8/14/1984	-	31.2	-	0.043
FLA20030073	8/27/1984	-	28	0.39	0.247
FLJXWQRR0005	10/23/1984	-	26	-	0.154
FLJXWQRR0005	12/5/1984	-	16.8	-	0.131
FLA20030073	1/14/1985	6.98	13.5	0.32	0.125
FLJXWQRR0005	2/19/1985	-	14	-	0.157
FLJXWQRR0005	4/10/1985	-	20.9	-	0.085
FLJXWQRR0005	6/11/1985	-	30.6	-	0.287
FLJXWQRR0005	10/15/1985	-	25.6	-	0.167
FLJXWQRR0005	12/3/1985	-	20.8	-	0.309
FLJXWQRR0005	2/26/1986	-	14	-	0.101
FLJXWQRR0005	5/21/1986	-	24	-	0.209

Station	Sample Date	CHLAC (µg/L))	TEMP (°C)	TN (mg/L)	TP (ma/L)
FLJXWQRR0005	7/8/1986	-	30	-	0.202
FLJXWQRR0005	9/9/1986	-	28.5	-	0.094
FLJXWQRR0005	11/4/1986	-	22	-	0.128
FLJXWQRR0005	2/4/1987	-	-	-	0.141
FLJXWQRR0005	3/31/1987	-	16.5	-	0.351
FLJXWQRR0005	6/2/1987	-	28.5	-	0.150
FLJXWQRR0005	9/15/1987	-	30.5	-	0.179
FLJXWQRR0005	1/12/1988	-	11	-	0.132
FLJXWQRR0005	5/24/1988	-	27	-	0.201
FLJXWQRR0005	8/9/1988	-	30.5	1.15	0.096
FLJXWQRR0005	1/25/1989	-	15.07	1.17	0.182
FLJXWQRR0005	3/14/1989	-	18	0.27	0.068
FLJXWQRR0005	5/17/1989	-	26.51	1.10	0.110
FLJXWQRR0005	8/16/1989	-	25.5	1.04	0.330
FLJXWQRR0005	10/25/1989	-	20.54	1.27	0.280
FLJXWQRR0005	3/26/1990	-	23.56	1.62	0.170
FLJXWQRR0005	6/19/1990	-	25.9	0.91	0.110
FLJXWQRR0005	8/22/1990	-	30.62	1.58	0.210
FLJXWQRR0005	11/6/1990	-	22.58	0.51	0.170
FLJXWQRR0005	1/15/1991	-	16.77	0.95	0.030
FLJXWQRR0005	3/26/1991	-	24.18	1.32	0.090
FLSJWMLSJ090	5/13/1992	25.39	24.1	1.02	0.190
FLSJWMLSJ090	8/6/1992	71.63	29.1	0.62	0.130
FLSJWMLSJ090	11/4/1992	13.9	23.1	1.18	0.150
FLSJWMLSJ090	1/12/1993	1.87	17.7	1.19	0.130
FLSJWMARLRM.S	6/9/1993	4.81	29.7	0.97	0.110
FLSJWMARLRM.S	7/8/1993	19.2	30.5	-	0.120
FLSJWMARLRM.S	8/24/1993	-	30.2	1.01	0.150
FLSJWMARLRM.S	9/21/1993	-	29.2	0.41	0.110
FLSJWMARLRM.S	10/7/1993	-	25.3	0.85	0.130
FLSJWMARLRM	10/31/1993	-	19.2	1.46	0.327
FLSJWMARLRM.S	12/21/1993	-	14.08	1.13	0.140
FLSJWMARLRM	1/31/1994	-	14.34	1.08	0.205
FLSJWMARLRM	3/2/1994	-	18.91	-	0.147
FLSJWMARLRM.S	3/29/1994	-	22.36	0.95	0.120
FLSJWMARLRM.S	5/4/1994	-	25.69	0.18	0.120
FLSJWMARLRM	7/1/1994	-	29.47	1.71	0.178
FLSJWMARLRM	9/12/1994	-	25.53	1.26	0.187
FLSJWMARLRM.S	10/3/1994	-	25.61	1.44	0.180

Station	Sample Date	CHLAC (µg/L))	TEMP (°C)	TN (mg/L)	TP (mg/L)
FLJXWQARLRM	10/11/1994	-	23.41	0.60	0.120
FLJXWQARLRM	11/16/1994	-	21.85	0.97	0.091
FLJXWQARLRM	4/6/1995	-	19.75	-	-
FLJXWQARLRM	6/5/1995	-	27.13	0.73	0.102
FLJXWQARLRM	7/18/1995	-	26.96	0.67	0.102
FLJXWQARLRM	8/25/1995	-	26.51	0.81	0.248
FLJXWQARLRM	10/10/1995	-	25.56	0.92	0.160
FLJXWQJAXSJR20A	1/12/1999	-	17	0.64	-
FLJXWQJAXSJR20A	2/10/1999	-	23	0.51	-
FLJXWQJAXSJR20A	3/10/1999	-	17.32	-	-
FLJXWQJAXSJR20A	4/20/1999	-	23.5	0.73	-
FLJXWQJAXSJR20A	5/12/1999	-	25.84	0.59	-
FLJXWQJAXSJR20A	6/30/1999	-	31.5	0.71	-
FLJXWQJAXSJR20A	7/14/1999	-	31.25	0.57	-
FLJXWQJAXSJR20A	8/17/1999	-	32.08	0.64	-
FLJXWQJAXSJR20A	10/13/1999	-	28.1	1.83	-
FLJXWQJAXSJR20A	11/8/1999	-	27	1.20	-
FLJXWQJAXSJR20A	12/1/1999	-	11.8	0.92	-
FLJXWQJAXSJR20A	1/12/2000	-	26.4	1.56	-
FLJXWQJAXSJR20A	2/9/2000	-	23.6	1.73	-
FLJXWQJAXSJR20A	3/8/2000	-	27.8	1.21	-
FLJXWQJAXSJR20A	4/12/2000	-	23.5	1.31	-
FLJXWQJAXSJR20A	5/9/2000	-	24.44	2.53	-
FLJXWQJAXSJR20A	6/13/2000	-	28.97	0.67	-
FLJXWQJAXSJR20A	7/19/2000	-	29.48	0.31	-
FLJXWQJAXSJR20A	8/15/2000	-	31	-	-
FLJXWQJAXSJR20A	9/13/2000	-	32	1.18	-
FLJXWQJAXSJR20A	10/24/2000	-	22	0.71	-
FLJXWQJAXSJR20A	11/8/2000	-	26	1.04	-
FLJXWQJAXSJR20A	12/12/2000	-	21	0.95	-
FLJXWQJAXSJR20A	1/9/2001	-	7	1.21	-
FLJXWQJAXSJR20A	2/14/2001	-	21	0.91	-
FLJXWQJAXSJR20A	3/14/2001	-	20	0.70	-
FLJXWQJAXSJR20A	4/11/2001	-	24.92	0.77	-
FLJXWQJAXSJR20A	5/9/2001	-	24.07	-	-
FLJXWQJAXSJR20A	6/6/2001	-	30.84	0.95	-
FLJXWQJAXSJR20A	7/10/2001	-	31.99	0.62	-
FLJXWQJAXSJR20A	8/8/2001	-	19.21	0.98	-
FLJXWQJAXSJR20A	9/19/2001	-	24.18	1.06	-

Station	Sample Date	CHLAC (μg/L))	TEMP (°C)	TN (mg/L)	TP (mg/L)
FLJXWQJAXSJR20A	10/9/2001	-	23.54	1.35	-
FLJXWQJAXSJR20A	11/19/2001	-	20.68	0.94	-
FLJXWQJAXSJR20A	12/18/2001	-	21.68	1.84	-
FLA20030073	3/21/2002	44	24.1	1.22	0.099
FLA20030073	4/17/2002	1	26.9	1.32	0.120
FLA20030073	5/6/2002	2.6	28.2	1.41	0.160
FLA20030073	6/19/2002	1	27.8	-	-
FLA20030073	7/15/2002	17	30.1	0.98	0.130
FLA20030073	8/12/2002	1	28.3	0.82	0.110
FLA20030073	9/4/2002	1	28.6	1.14	0.130
FLA20030073	10/16/2002	1	24.7	1.11	0.120
FLA20030073	11/6/2002	1	22.8	2.02	0.270
FLA20030073	12/3/2002	1	15.2	1.43	0.130
FLA20030073	2/27/2007	1	18.8	0.66	0.096
FLA20030811	2/27/2007	1	17.8	0.96	0.110
FLA20030812	2/27/2007	1	18.3	0.66	0.091
FLA20030813	2/27/2007	1	18.8	0.69	0.110
FLA20030073	4/25/2007	1	25.1	0.75	0.094
FLA20030811	4/25/2007	1	23	0.63	0.082
FLA20030812	4/25/2007	1	23.8	0.74	0.091
FLA20030813	4/25/2007	10	23.6	0.89	0.110
FLA20030073	8/28/2007	10	30.9	1.21	0.210
FLA20030811	8/28/2007	5.1	31.5	1.25	0.210
FLA20030812	8/28/2007	8.7	31.3	1.17	0.210
FLA20030813	8/28/2007	12	30.9	1.19	0.200
FLA20030073	10/17/2007	15	25.6	1.58	0.170
FLA20030811	10/17/2007	7.9	25.4	1.44	0.180
FLA20030812	10/17/2007	9.9	25.5	1.55	0.150
FLA20030813	10/17/2007	16	25.4	1.70	0.170
FLA20030073	12/4/2007	15	17.5	1.49	0.140
FLA20030811	12/4/2007	16	17.6	1.74	0.150
FLA20030812	12/4/2007	12	17.2	1.57	0.140
FLA20030813	12/4/2007	15	17.7	1.49	0.150
FLA20030073	2/14/2008	-	16.9	1.09	0.110

Appendix C: Kruskal–Wallis Analysis of CHLAC, TN, TP, Conductance, and Chloride Observations versus Season in the Arlington River (WBID 2265A)

Kruskal-Wallis One-Way Analysis of Variance for 38 cases Dependent variable is VCHLAC Grouping variable is SEASON\$

Group	Count	Rank Sum
FALL	12	267.000
SPRING	10	165.000
SUMMER	9	208.000
WINTER	7	101.000

Kruskal-Wallis Test Statistic = 4.078 Probability is 0.253 assuming Chi-square distribution with 3 df

Kruskal-Wallis One-Way Analysis of Variance for 137 cases Dependent variable is VTN Grouping variable is SEASON\$

Group Count Rank Sum

FALL	41	3346.500
SPRING	30	1631.500
SUMMER	34	2171.000
WINTER	32	2304.000

Kruskal-Wallis Test Statistic = 8.969 Probability is 0.030 assuming Chi-square distribution with 3 df

Kruskal-Wallis One-Way Analysis of Variance for 123 cases Dependent variable is VTP Grouping variable is SEASON\$

Group Count Rank Sum

FALL	34	2353.000
SPRING	26	1353.500
SUMMER	34	2319.500
WINTER	29	1600.000

Kruskal-Wallis Test Statistic = 5.515 Probability is 0.138 assuming Chi-square distribution with 3 df Kruskal-Wallis One-Way Analysis of Variance for 164 cases

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Dependent variable is VCONDUCTANCE Grouping variable is SEASON\$

FALL	48	2880.500
SPRING	37	4127.000
SUMMER	39	3344.000
WINTER	40	3178.500

Kruskal-Wallis Test Statistic = 24.951 Probability is 0.000 assuming Chi-square distribution with 3 df

Kruskal-Wallis One-Way Analysis of Variance for 176 cases Dependent variable is VCHLORIDE Grouping variable is SEASON\$

Group	Count	Rank Sum
FALL	55	3602.000
SPRING	45	4811.000
SUMMER	36	3522.000
WINTER	40	3641.000

Kruskal-Wallis Test Statistic = 18.400 Probability is 0.000 assuming Chi-square distribution with 3 df

Appendix D: Kruskal–Wallis Analysis of CHLAC, TN, TP, Conductance, and Chloride Observations versus Month in the Arlington River (WBID 2265A)

Kruskal-Wallis One-Way Analysis of Variance for 38 cases Dependent variable is VCHLAC Grouping variable is MONTH

Group	Со	unt Rank Sum
1	2	34.000
2	4	30.000
3	1	37.000
4	6	89.500
5	2	51.000
6	2	24.500
7	2	67.000
8	6	133.500
9	1	7.500
10	5	110.000
11	2	34.500
12	5	122.500

Kruskal-Wallis Test Statistic = 16.593 Probability is 0.120 assuming Chi-square distribution with 11 df

Kruskal-Wallis One-Way Analysis of Variance for 137 cases Dependent variable is VTN Grouping variable is MONTH

Count Rank Sum

1	11	857.000
2	9	493.500
3	12	953.500
4	12	670.000
5	8	568.500
6	10	393.000
7	8	436.000
8	18	1205.500
9	8	529.500
10	17	1440.500
11	11	753.500
12	13	1152.500

Group

Kruskal-Wallis Test Statistic = 16.491 Probability is 0.124 assuming Chi-square distribution with 11 df

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Kruskal-Wallis One-Way Analysis of Variance for 123 cases Dependent variable is VTP Grouping variable is MONTH

Group	Co	unt Rank Sum
1	10	638.500
2	8	298.000
3	11	663.500
4	9	421.500
5	9	615.500
6	8	316.500
7	8	565.500
8	17	1230.500
9	9	523.500
10	15	1098.500
11	7	431.500
12	12	823.000

Kruskal-Wallis Test Statistic = 12.926 Probability is 0.298 assuming Chi-square distribution with 11 df

Kruskal-Wallis One-Way Analysis of Variance for 164 cases Dependent variable is VCONDUCTANCE Grouping variable is MONTH

Group	Co	unt	Rank Sum
1	12	8	56.500
2	15	12	71.000
3	13	10	51.000
4	13	13	61.500
5	12	12	72.000
6	12	14	93.500
7	12	9	90.000
8	18	17	42.000
9	9	6	12.000
10	20	11	58.500
11	12	8	04.500
12	16	9	17.500

Kruskal-Wallis Test Statistic = 29.450 Probability is 0.002 assuming Chi-square distribution with 11 df

Florida Department of Environmental Protection

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Kruskal-Wallis One-Way Analysis of Variance for 176 cases Dependent variable is VCHLORIDE Grouping variable is MONTH

Group	Co	unt Rank Sum
1	10	838.000
2	20	1865.500
3	10	937.500
4	18	1646.000
5	8	974.000
6	19	2191.000
7	7	721.000
8	21	2174.500
9	8	626.500
10	26	1957.000
11	9	707.000
12	20	938.000

Kruskal-Wallis Test Statistic = 27.246 Probability is 0.004 assuming Chi-square distribution with 11 df

Appendix E: Chart of CHLAC, TN, Nitrogen Ammonia (NH4), Nitrate Nitrite (NO3O2), and TP Observations by Season, Station, and Year in the Arlington River (WBID 2265A)

CHLAC BY STATION





TOTAL NITROGEN BY STATION

AMMONIA BY STATION



NO3NO2 BY STATION



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TOTAL PHOSPHORUS BY STATION





CHLAC BY YEAR

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TOTAL NITROGEN BY YEAR









NO3NO2 BY YEAR

TOTAL PHOSPHORUS BY YEAR



CUMULATIVE FREQUENCY PLOT CHLAC



CUMULATIVE FREQUENCY PLOT TN



CUMULATIVE FREQUENCY PLOT NO3NO2



CUMULATIVE FREQUENCY PLOT NH4



CUMULATIVE FREQUENCY PLOT TP




Appendix F: Chart of Rainfall for JIA, 1948–2008

Appendix G: Spearman Correlation Matrix Analysis for Water Quality Parameters in the Arlington River (WBID 2265A)

Spearman correlation matrix

	PRECIP	PRECIP3D	PRECIP7D	PRECIP14	JULIANDATE	VBOD	VCHLAC
PRECIP	1						
PRECIP3DAY	0.555	1					
PRECIP7DAY	0.443	0.673	1				
PRECIP14DAY	0.347	0.441	0.662	1			
JULIANDATE	0.004	-0.112	-0.189	-0.233	1		
VBOD	0.02	0	-0.131	-0.145	0.429	1	
VCHLAC	0	0.325	0.177	0.233	-0.005	0.281	1
VCHLOR	-0.094	-0.194	-0.121	-0.294	0.306	0.248	-0.442
VCOLOR	0.017	-0.06	-0.02	0.15	-0.3	-0.156	0.439
VCOND	-0.138	-0.182	-0.169	-0.307	0.148	0.258	-0.352
VDO	-0.11	-0.231	-0.238	-0.238	0.216	0.122	-0.08
VDOSAT	-0.093	-0.22	-0.219	-0.194	0.316	0.312	-0.049
VNH4	0.016	0.098	0.142	0.226	-0.323	-0.236	0.091
VN0302	-0.088	-0.067	-0.082	-0.09	0.154	-0.061	0.117
VPH	-0.145	-0.187	-0.334	-0.283	0.183	0.149	-0.058
VSD	-0.088	-0.167	-0.085	-0.211	0.045	-0.007	-0.33
VSALIN	-0.193	-0.17	-0.238	-0.383	-0.017	0.205	-0.304
VSO4	-0.175	-0.143	-0.196	-0.337	0.063	0.196	-0.403
VTEMP	0.124	0.103	0.116	0.162	0.109	0.158	0.134
VTKN	-0.062	-0.141	-0.187	-0.001	0.218	0.268	0.327
VTN	-0.081	-0.145	-0.18	0.015	-0.121	0.155	0.326
VTOC	0	-0.004	0.091	0.213	-0.422	-0.671	0.265
VTORTH	-0.023	0.02	0.053	0.083	-0.175	-0.211	1
VTP	0.095	0.199	0.052	0.037	-0.173	0.101	0.46
VTSS	-0.069	-0.024	-0.044	0.007	-0.315	0.154	0.225
VTURB	0.149	0.144	0.071	0.176	0.121	-0.084	0.455

	VCHLOR	VCOLOR	VCOND	VDO	VDOSAT	VNH4	VN0302
VCHLOR	1						
VCOLOR	-0.564	1					
VCOND	0.887	-0.701	1				
VDO	0.141	0.036	-0.039	1			
VDOSAT	0.359	-0.164	0.146	0.877	1		
VNH4	-0.25	0.498	-0.203	-0.286	-0.413	1	
VN0302	-0.341	0.491	-0.253	-0.106	-0.373	0.199	1
VPH	0.383	-0.332	0.338	0.274	0.319	-0.183	-0.252
VSD	0.524	-0.651	0.469	-0.251	-0.171	-0.187	-0.266
VSALIN	0.945	-0.821	0.974	-0.102	0.075	-0.159	-0.277
VSO4	0.962	-0.887	0.826	0.099	0.226	-0.501	-0.412
VTEMP	0.289	-0.269	0.233	-0.437	-0.033	-0.046	-0.299
VTKN	-0.334	0.482	-0.338	-0.013	-0.083	0.128	0.346
VTN	-0.381	0.515	-0.367	-0.066	-0.213	0.249	0.591
VTOC	-0.431	0.702	-0.381	0.008	-0.178	0.289	0.352
VTORTH	-0.189	0.103	-0.131	-0.257	-0.277	0.135	0.26
VTP	-0.186	0.374	-0.046	-0.159	-0.23	0.244	0.385
VTSS	0.135	0.088	0.093	0.174	0.212	-0.074	-0.04
VTURB	-0.501	0.717	-0.453	0.065	0.046	0.14	0.112
				LOO1			
		VSD	VSALIN	VSO4		VIKN	VIN
	1						
VSD	0.094	1					
VSALIN	0.289	0.534	1				
VS04	0.541	0.465	0.988	1			
VTEMP	-0.018	0.148	0.256	0.374	1		
VTKN	-0.017	-0.372	-0.382	-0.574	0.03	1	
VTN	-0.069	-0.381	-0.379	-0.573	-0.049	0.943	1
VTOC	-0.258	-0.421	-0.629	-0.572	-0.137	0.518	0.576
VTORTH	-0.031	-0.049	0.103	-0.527	0.04	0.07	0.275
VTP	-0.13	-0.167	-0.015	-0.319	0.034	0.375	0.391
VTSS	-0.033	-0.465	0.097	0.119	0.053	0.371	0.326
VTURB	-0.116	-0.696	-0.534	-0.76	-0.047	0.375	0.352

Spearman correlation matrix (cont.)

	VTOC	VTORTH	VTP	VTSS	VTURB
VTOC	1				
VTORTH	0	1			
VTP	0.159	0.488	1		
VTSS	0.129	0.139	0.177	1	
VTURB	0.529	0.097	0.274	0.32	1

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	PRECIP	PRECIP3D	PRECIP7D	PRECIP14	JULIANDATE	VBOD	VCHLAC
PRECIP	271						
PRECIP3DAY	271	271					
PRECIP7DAY	271	271	271				
PRECIP14DAY	271	271	271	271			
JULIANDATE	271	271	271	271	271		
VBOD	137	137	137	137	137	137	
VCHLAC	38	38	38	38	38	22	38
VCHLOR	176	176	176	176	176	118	25
VCOLOR	110	110	110	110	110	96	28
VCOND	164	164	164	164	164	71	38
VDO	256	256	256	256	256	133	38
VDOSAT	240	240	240	240	240	126	38
VNH4	169	169	169	169	169	74	37
VN0302	124	124	124	124	124	66	36
VPH	248	248	248	248	248	125	37
VSD	81	81	81	81	81	49	38
VSALIN	108	108	108	108	108	55	32
VSO4	38	38	38	38	38	27	24
VTEMP	249	249	249	249	249	131	38
VTKN	102	102	102	102	102	64	37
VTN	137	137	137	137	137	72	36
VTOC	54	54	54	54	54	28	32
VTORTH	59	59	59	59	59	42	2
VTP	123	123	123	123	123	43	37
VTSS	152	152	152	152	152	73	27
VTURB	119	119	119	119	119	70	28

	VCHLOR	VCOLOR	VCOND	VDO	VDOSAT	VNH4	VN0302
VCHLOR	176						
VCOLOR	103	110					
VCOND	106	57	164				
VDO	172	107	162	256			
VDOSAT	162	98	152	240	240		
VNH4	119	59	150	162	151	169	
VN0302	106	60	111	120	115	122	124
VPH	167	104	158	243	228	159	114
VSD	55	46	78	78	78	72	70
VSALIN	65	42	87	108	108	82	81
VSO4	38	36	35	38	33	37	35
VTEMP	169	106	157	244	239	160	123
VTKN	84	59	94	99	95	99	100
VTN	92	58	123	131	122	135	101
VTOC	39	32	51	53	47	52	41
VTORTH	52	33	53	56	50	57	50
VTP	83	41	107	119	112	117	90
VTSS	117	58	135	145	134	149	111
VTURB	110	60	106	115	110	116	113
	VPH	VSD	VSALIN	VSO4	VTEMP	VTKN	VTN
VPH	248						
VSD	74	81					
VSALIN	105	65	108				
VSO4	37	25	22	38			
VTEMP	234	81	108	38	249		
VTKN	93	72	80	35	102	102	
VTN	128	72	79	35	130	100	137
VTOC	52	34	29	31	51	40	50
VTORTH	51	32	35	16	58	51	54
VTP	119	44	53	36	118	69	94
VTSS	143	61	70	36	143	90	118
VTURB	110	62	71	36	118	92	94

Pairwise frequency table (cont.)

	VTOC	VTORTH	VTP	VTSS	VTURB
VTOC	54				
VTORTH	9	59			
VTP	48	21	123		
VTSS	41	56	104	152	
VTURB	34	54	84	117	119

Appendix H: Linear Regression Analysis of CHLAC Observations versus Nutrients and Rainfall in the Arlington River (WBID 2265A)

Dep Var: VCHLAC N: 38 Multiple R: 0.157 Squared multiple R: 0.025

Adjusted squared multiple R: 0.000 Standard error of estimate: 14.288

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	0.250	11.325	0.000		0.022	0.982
VTEMP	0.440	0.461	0.157	1.000	0.954	0.347

Analysis of Variance

Source		Sum-of-Squa	res df	Mean-Squa	re	F-ratio	Р
Regression	า	185.716	1	185.716		0.910	0.347
Residual		7349.553	36	204.154			
*** WARNI	NG ***						
Case	110 has la	arge leverage	(Leverage	e = 0.294)			
Case	159 is an	outlier (St	udentized	Residual =	5.834)		
Case	226 has la	arge leverage	(Leverage	e = 0.329)			
Durbin-Wa	tson D Sta	tistic 1.3	12				

First Order Autocorrelation 0.292

Dep Var: VCHLAC N: 36 Multiple R: 0.341 Squared multiple R: 0.117

Adjusted squared multiple R: 0.034 Standard error of estimate: 14.263

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	-56.222	34.047	0.000		-1.651	0.108
VTEMP	3.160	1.562	1.105	0.093	2.024	0.051
VTN	51.930	29.508	1.353	0.047	1.760	0.088
VTEMP*VTN	-2.473	1.358	-1.719	0.031	-1.821	0.078

Analysis of Variance

Source	Sum-of-Squares	s df	Mean-Squar	e F-ratio	Р
Regression	858.456	3	286.152	1.407	0.259
Residual	6510.090	32	203.440		
*** WARNI	NG ***				
Case	80 has large leverage	(Leverage =	1.506)		
Case	104 has large leverage	(Leverage =	0.447)		
Case	110 has large leverage	(Leverage =	7.509)		
Case	111 has large leverage	(Leverage =	0.523)		
Case	112 has large leverage	(Leverage =	0.520)		
Case	116 has large leverage	(Leverage =	0.430)		
Case	126 has large leverage	(Leverage =	0.389)		
Case	129 has large leverage	(Leverage =	0.578)		
Case	159 is an outlier (St	udentized Re	sidual =	5.393)	
Case	165 has large leverage	(Leverage =	0.466)		

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Case	172 has large leverage	(Leverage =	0.388)
Case	209 has large leverage	(Leverage =	0.433)
Case	210 has large leverage	(Leverage =	0.401)
Case	218 has large leverage	(Leverage =	0.589)
Case	220 has large leverage	(Leverage =	0.616)
Case	226 has large leverage	(Leverage =	0.374)
Case	232 has large leverage	(Leverage =	0.419)
Durale in A	Nata an D Chatiatia d O	00	

Durbin-Watson D Statistic1.282First Order Autocorrelation0.304

Dep Var: VCHLAC N: 37 Multiple R: 0.389 Squared multiple R: 0.152

Adjusted squared multiple R: 0.074 Standard error of estimate: 13.827

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	-74.599	44.197	0.000		-1.688	0.101
VTEMP	3.292	1.874	1.173	0.058	1.757	0.088
VTP	536.955	300.905	2.306	0.015	1.784	0.084
VTEMP*VTP	-20.169	12.501	-2.501	0.011	-1.613	0.116

Analysis of Variance

Source	Sum-of-Squares	df	Mean-Squa	re F-ratio	Р
Regression	1127.094	3	375.698	1.965	0.138
Residual	6309.075	33	191.184		
*** WARNI	NG ***				
Case	61 has large leverage	(Leverage =	0.827)		
Case	63 has large leverage	(Leverage =	0.753)		
Case	70 has large leverage	(Leverage =	4.470)		
Case	73 has large leverage	(Leverage =	0.373)		
Case	76 has large leverage	(Leverage =	13.004)		
Case	89 has large leverage	(Leverage =	10.284)		
Case	90 has large leverage	(Leverage =	4.293)		
Case	94 has large leverage	(Leverage =	0.878)		
Case	108 has large leverage	(Leverage =	• 0.726)		
Case	110 has large leverage	(Leverage =	= 2.439)		
Case	111 has large leverage	(Leverage =	• 0.503)		
Case	112 has large leverage	(Leverage =	= 0.487)		
Case	116 has large leverage	(Leverage =	• 0.911)		
Case	121 has large leverage	(Leverage =	= 1.761)		
Case	123 has large leverage	(Leverage =	• 0.662)		
Case	125 has large leverage	(Leverage =	= 0.730)		
Case	132 has large leverage	(Leverage =	= 0.841)		
Case	134 has large leverage	(Leverage =	= 0.507)		
Case	141 has large leverage	(Leverage =	= 2.478)		
Case	151 has large leverage	(Leverage =	= 0.396)		
Case	156 has large leverage	(Leverage =	• 0.635)		
Case	159 is an outlier (S	tudentized Re	esidual =	6.345)	
Case	167 has large leverage	(Leverage =	• 0.995)		
Case	169 has large leverage	(Leverage =	= 0.492)		

Durbin-Watson D Statistic 1.537

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First Order Autocorrelation 0.228

Dep Var: VCHLAC N: 36 Multiple R: 0.552 Squared multiple R: 0.304

Adjusted squared multiple R: 0.160 Standard error of estimate: 13.295

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	-126.785	49.848	0.000		-2.543	0.017
VTEMP	4.692	2.022	1.640	0.048	2.321	0.028
VTP	654.909	379.198	2.817	0.009	1.727	0.095
VTN	62.910	32.252	1.639	0.034	1.951	0.061
VTEMP*VTP	-14.986	13.732	-1.867	0.008	-1.091	0.284
VTEMP*VTN	-2.124	1.445	-1.476	0.024	-1.470	0.152
VTP*VTN	174.752	121.019	-1.340	0.028	-1.444	0.159

Analysis of Variance

Source	Sum-of-Square	s df	Mean-Sq	uare	F-ratio	Р
Regressior	n 2242.816	6	373.803		2.115	0.082
Residual	5125.730	29	176.749			
*** WARNI	NG ***					
Case	61 has large leverage	(Leverage =	0.903)			
Case	63 has large leverage	(Leverage =	3.193)			
Case	70 has large leverage	(Leverage =	7.052)			
Case	73 has large leverage	(Leverage =	1.535)			
Case	83 has large leverage	(Leverage =	1.466)			
Case	89 has large leverage	(Leverage =	13.774)			
Case	94 has large leverage	(Leverage =	1.006)			
Case	97 has large leverage	(Leverage =	0.840)			
Case	100 has large leverage	(Leverage =	1.090)			
Case	104 has large leverage	(Leverage =	0.725)			
Case	108 has large leverage	(Leverage =	0.775)			
Case	110 has large leverage	(Leverage =	16.919)			
Case	111 has large leverage	(Leverage =	2.598)			
Case	112 has large leverage	(Leverage =	1.169)			
Case	116 has large leverage	(Leverage =	0.979)			
Case	121 has large leverage	(Leverage =	3.000)			
Case	123 has large leverage	(Leverage =	0.859)			
Case	126 has large leverage	(Leverage =	1.331)			
Case	129 has large leverage	(Leverage =	0.603)			
Case	148 has large leverage	(Leverage =	0.631)			
Case	150 has large leverage	(Leverage =	0.519)			
Case	156 has large leverage	(Leverage =	0.776)			
Case	159 is an outlier (St	tudentized Resi	dual =	6.358)		
Case	165 has large leverage	(Leverage =	0.560)			
Case	167 has large leverage	(Leverage =	1.124)			
Case	169 has large leverage	(Leverage =	0.649)			
Case	238 is an outlier (St	tudentized Resi	dual =	3.199)		
Case	246 has large leverage	(Leverage =	0.893)			
		72				

Durbin-Watson D Statistic1.913First Order Autocorrelation0.040

Dep Var: VCHLAC N: 38 Multiple R: 0.578 Squared multiple R: 0.334

Adjusted squared multiple R: 0.316 Standard error of estimate: 11.803

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	7.529	2.065	0.000		3.646	0.001
PRECIP3DAY	11.125	2.616	0.578	1.000	4.253	0.000

Analysis of Variance

Source	Sum-of-Squares	df	Mean-Squ	lare	F-ratio	Р
Regression	2520.148	1	2520.14	18	18.090	0.000
Residual	5015.122	36	139.30)9		
*** WARNI	NG ***					
Case	32 has large leverage	(Levera	age = 0	.559)		
Case	69 has large leverage	(Levera	age = 0	.298)		
Case	89 has large leverage	(Levera	age = 0	.261)		
Case	158 has large leverage	(Lever	age = ().647)		
Case	158 is an outlier (St	udentiz	ed Residua	=	-4.359)	
Case	158 has large influence	(Cook	distance =	11	.587)	
Case	159 has large leverage	(Lever	age = ().285)		
Case	159 is an outlier (St	udentiz	ed Residua	=	4.314)	
Case	167 has large leverage	(Lever	age = ().718)		
Case	175 has large leverage	(Lever	age = 0).285)		
Case	176 has large leverage	(Lever	age = 1	.803)		
Case	180 has large leverage	(Lever	age = ().521)		
Case	181 has large leverage	(Lever	age = 1	.336)		
Case	196 has large leverage	(Lever	age = 0).744)		

Durbin-Watson D Statistic	1.743
First Order Autocorrelation	0.055

Dep Var: VCHLAC N: 38 Multiple R: 0.565 Squared multiple R: 0.319

Adjusted squared multiple R: 0.300 Standard error of estimate: 11.939

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	7.179	2.130	0.000		3.370	0.002
PRECIP7DAY	6.762	1.647	0.565	1.000	4.107	0.000

Analysis of Variance

Source	Sum-of-Squares	df	Mean-S	Square	F-ratio	Р
Regression	n 2403.922	1	2403.	922	16.865	0.000
Residual	5131.347	36	142.	537		
*** WARN	NG ***					
Case	14 has large leverage	(Lever	age =	0.429)		
Case	15 has large leverage	(Lever	age =	0.429)		
Case	20 has large leverage	(Lever	age =	0.388)		
Case	21 has large leverage	(Lever	age =	0.388)		
Case	51 has large leverage	(Lever	age =	1.324)		
Case	52 has large leverage	(Lever	age =	1.324)		
Case	69 has large leverage	(Lever	age =	0.349)		
Case	86 has large leverage	(Lever	age =	0.481)		
Case	158 has large leverage	(Leve	rage =	0.303)		
Case	159 has large leverage	(Leve	rage =	0.478)		
Case	159 is an outlier (St	udentiz	ed Resid	ual =	3.779)	
Case	167 has large leverage	(Leve	rage =	0.310)		
Case	176 has large leverage	(Leve	rage =	0.689)		
Case	181 has large leverage	(Leve	rage =	0.850)		
Case	194 has large leverage	(Leve	rage =	0.316)		
Case	196 has large leverage	(Leve	rage =	0.335)		
Case	234 has large leverage	(Leve	rage =	0.427)		
Case	238 is an outlier (St	udentiz	ed Resid	ual =	3.087)	
Durbin-Wa	tson D Statistic 1.3	83				
First Order	Autocorrelation 0.22	27				

Appendix I: Monthly and Annual Precipitation at JIA, 1955–2008

Rainfall is in inches, and represents data from JIA.

													Annual
Year	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Total
1955	3.1	2.46	1.66	1.5	4.51	2.7	5.53	3.85	10.6	5.36	1.9	0.2	43.33
1956	2.9	2.94	0.81	2.33	3.98	7.87	8.25	5.24	2.89	13.4	0.4	0	51.08
1957	0.3	1.69	3.87	1.61	5.25	7.1	12.3	3.3	8.33	3.5	1.6	1.3	50.18
1958	3.4	3.74	3.38	8.24	3.79	3.96	4.37	4.67	4.75	5.07	2	2.8	50.14
1959	3	5.22	9.75	2.65	9.2	2.94	4.51	2.86	5.67	3.12	2.2	1	52.08
1960	2.1	5.17	6.94	3.54	1.18	4.7	16.2	6.5	8.57	2.95	0.1	1.5	59.45
1961	2.9	4.85	1.17	4.16	3.06	5.27	3.48	10.6	1.02	0.27	0.9	0.5	38.15
1962	2.2	0.52	3.1	2.36	1.12	8.22	6.31	10.1	4.37	1.13	2.1	2.5	43.9
1963	5.4	6.93	2.23	1.75	1.74	12.5	6.47	4.95	4.88	1.53	2.7	3.6	54.66
1964	7.3	6.55	1.76	4.65	4.8	4.67	6.12	5.63	10.3	5.09	3.3	4.8	65.03
1965	0.7	5.5	3.91	0.95	0.94	9.79	2.71	9.58	11	1.75	1.9	3.8	52.47
1966	4.6	5.97	0.71	2.25	10.4	7.74	11.1	3.88	5.94	1.38	0.2	1.1	55.3
1967	3.1	4.35	0.81	2	1.18	12.9	5.22	12.3	1.8	1.13	0.2	4.7	49.68
1968	0.8	3.05	1.2	0.99	2.17	12.3	6.84	16.2	2.68	5.09	1.3	1.1	53.72
1969	0.8	3.39	4.23	0.34	3.78	5.12	5.89	15.1	10.3	9.81	4.6	3.9	67.26
1970	4.2	8.85	9.98	1.77	1.84	2.65	7.6	11	3.2	3.95	0	1.6	56.55
1971	2	2.55	2.41	4.07	1.9	5.52	5.07	12.8	4.17	6.46	0.8	5.9	53.69
1972	5.8	3.48	4.43	2.98	8.26	6.75	3.15	9.76	2.6	4.46	4.2	1.4	57.29
1973	4.6	5.07	10.2	11.6	5.33	4.1	5.45	7.49	7.86	4.08	0.4	4.3	70.57
1974	0.3	1.28	3.47	1.53	4.14	5.53	9.83	11.2	8.13	0.34	1	1.7	48.52
1975	3.5	2.58	2.46	5.78	7	5.21	6.36	6.23	5.24	3.63	0.4	1.8	50.15
1976	2.3	1.05	3.41	0.63	10	4.26	5.41	6.37	8.56	1.63	2.4	4.8	50.87
1977	3	3.24	1.03	1.76	3.07	2.65	1.97	7.26	7.45	1.68	3.1	3.4	39.56
1978	4.6	4.17	2.83	2.24	9.18	2.62	6.67	2.39	4.4	1.26	0.8	1.8	43.04
1979	6.3	3.75	1	4.18	7.54	5.91	4.67	4.78	17.8	0.25	3.6	2	61.76
1980	2.6	1.06	6.83	3.91	3.02	4.59	5.29	3.97	3.03	2.69	2.3	0.2	39.53
1981	0.9	4.53	5.41	0.32	1.48	3.31	2.46	6.47	1.22	1.35	4.9	3.4	35.77
1982	3	1.67	4.26	3.6	3.55	8.06	3.81	6.93	9.32	3.37	1.9	2	51.52
1983	7.2	4.27	8.46	4.65	1.38	6.86	6.11	4.63	4.61	4.29	3.3	6.4	62.19
1984	2.1	4.67	5.77	3.14	1.46	4.76	6.01	3.78	12.3	1.53	3.3	0.1	48.96
1985	1.1	1.45	1.26	2.76	2.08	3.71	6.33	8.93	16.8	8.34	2.1	3.6	58.39
1986	4.2	4.72	5.44	0.93	2.13	2.53	3.27	9.6	1.99	1.8	2.9	4.7	44.1
1987	4.1	6.47	6.27	0.14	0.75	4.18	4.4	4.48	7.13	0.3	5	0.2	43.39
1988	6.4	6.08	2.65	3.44	1.35	3.71	4.5	8.48	16.4	2.35	4.3	1.1	60.68
1989	1.7	1.77	2.14	2.79	1.55	3.66	8.98	9.16	14.4	1.39	0.5	3.4	51.45
1990	1.8	4.07	1.59	1.34	0.18	1.59	6.53	3.81	2.6	4.54	1.2	1.9	31.2
1991	10	1.52	7.33	6.31	9.35	11.7	15.9	3.48	6.2	6.36	0.7	0.6	79.63
1992	5.8	2.64	4.09	5.33	5.97	7.04	3.32	10.8	7.33	8.34	1.9	0.7	63.18
1993	3.9	2.89	5.98	0.85	1.6	2.52	7.54	2.96	7.6	8.84	3.6	1.9	50.12
1994	6.6	0.92	2.14	1.51	3.15	14	8.26	3.29	9.79	10.2	3.5	3.9	67.26
1995	1.9	2.07	3.67	1.77	1.77	5.35	9.45	9.93	5.41	3.53	3.2	2.2	50.25

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Year	Jan	Feb	Mar	Apr	Мау	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual Total
1996	1.1	1.11	6.83	2.85	0.72	11.4	4.2	7.83	8.49	11.5	1.4	3.2	60.63
1997	2.9	1.28	1.84	4.56	3.43	6.33	7.69	8.24	3.97	4.84	2.4	9.8	57.27
1998	3.5	11.1	2.64	4.71	0.96	2.95	7.29	10.1	7.65	3.01	2.4	0.4	56.72
1999	4.6	1.7	0.4	1.92	1.02	7.75	3.56	3.51	13	3.24	0.8	0.9	42.44
2000	2.8	1.17	1.79	2.6	1.15	2.43	5.69	7.38	11.6	0.23	1.6	1.4	39.77
2001	0.9	0.68	5.48	0.62	2.56	5.59	8.31	3.58	16	0.81	1.4	3.1	49.14
2002	4.5	0.82	4.38	2.41	0.47	6.24	7.8	8.14	9.31	2.58	2.7	5.4	54.72
2003	0.1	4.66	10.7	2.63	2.54	6.75	7.33	1.83	3.04	2.98	0.7	1.2	44.47
2004	1.6	4.47	1.36	2.02	1.24	17.2	8.6	9.85	16.3	1.32	2.9	2.7	69.47
2005	1.9	3.56	3.67	4.53	3.51	14.8	7.37	4.43	5.76	6.49	1.1	7.4	64.44
2006	2.30	3.91	0.68	1.22	2.01	7.25	3.97	7.08	4.55	1.81	0.39	2.90	38.07
2007	2.29	2.40	2.22	1.02	1.12	6.68	9.48	3.57	5.44	8.85	0.17	2.74	45.98
2008	2.63	5.22	3.50	2.34	0.66	8.21	8.73	16.83	5.84	1.62	1.01	0.59	46.01
AVG	3.21	3.54	3.81	2.82	3.29	6.37	6.55	6.99	7.43	3.87	1.98	2.62	52.32



Appendix J: Annual and Monthly Average Precipitation at JIA

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Appendix K: Response to Comments

August 4, 2009

John P. Pappas, P.E. Deputy Director Public Works City of Jacksonville 214 N. Hogan Street, Suite 1079 Jacksonville, FL 32202

Subject: Draft Total Maximum Daily Loads for the Ortega (WBID 2213P), Trout (WBID 2203), and Arlington (WBID 2265A) Rivers

Dear Mr. Pappas:

Thank you for providing comments on several of our draft Total Maximum Daily Load reports in your letter of July 15th. We appreciate you and your staff taking the time to help us improve the clarity of these reports and have incorporated your suggested changes wherever possible. As you will see below, your comments are reproduced in the order in which they were presented and then our responses are provided.

 Explain why upstream tributary areas to the WBIDs appear to exclude some areas7 (see Figure 4.4 for Arlington River, Figure 4.6 for Ortega River, Figure 4.5 for Trout River). These figures exclude large sections of upstream tributary area.

Department Response: The intent of these figures and load estimates was to provide an overview of relative contributions of nitrogen and phosphorus from various sources to the impaired WBID from upstream areas, but were not used to establish the Total Maximum Daily Load reductions. If the City could provide a GIS coverage of the upstream tributary drainage area for these WBIDs, the analysis could be revised. As noted in the documents, it was our understanding that the City has contracted with Camp Dresser & McKee, Inc. to model these sub-basins with the Watershed Management Model to develop discharge and pollutant load information as part of an update to the Master Stormwater Management Plan. Determination of the assimilative capacity described in Chapter 5 was based upon observed relationships between multiple water quality parameters in the impaired waterbody. While the source assessments described in Chapter 4 were not a factor in the development of nutrient reductions, they were included to aid in the development of the Basin Management Action Plan (reduction implementation) process.

2) The measured TN and TP concentrations for the Trout River appear to be higher than would be expected for a relatively undeveloped basin. The higher measured concentrations suggest that there is some load in the watershed that is not accounted for, and/ or the station is affected by high downstream TN and TP concentrations migrating upstream. A portion of this inconsistency could be due to tidal influences from the St Johns River. John P. Pappas, P.E. Deputy Director Public Works City of Jacksonville August 4, 2009 Page Two

Department Response: As noted in the document, there was a considerable increase in the median TN and TP medians in the Middle Trout River WBID (2203) compared to the upstream segment, i.e., the upper Trout River (WBID 2223). Based on data from the Impaired Waters Rule Run 35 database, the median TN and TP concentrations for WBID 2203 were 1.43 mg/L and 0.296 mg/L respectively, versus medians of 0.55 mg/L and 0.068 mg/L, respectively, in WBID 2223 over the same period. Over the same period, median TN and TP concentrations in the downstream Trout River WBID (2203A) were 0.93 mg/L and 0.13 mg/L, respectively. Regarding your concern over the possible effects of high downstream TN and TP concentrations migrating upstream, flow records from the USGS site (02246599) located near the bottom of the middle Trout River WBID did not reflect any negative flows over the period of record (10/1/2002 – 8/10/2006). Daily flows ranged between 0.04 and 130 cubic feet per second (CFS), with a mean flow of 4.9 CFS. A cumulative frequency plot of flow is presented below. As part of the county stormwater program and Phase 1 MS4 monitoring, has the City identified any possible sources that could help explain the elevated TN and TP levels in this WBID?

USGS GAGE 02246599



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> 3) For the Trout River, the relatively low estimated TP loads/ concentrations and the high measured TP concentrations indicate an inconsistency between the modeling and observed conditions. The TMDL for the Trout River should be revisited to account for the additional nutrient source.

Department Response: As noted in our response to Item #1, the load/concentration estimates described in Chapter 4 were provided to indicate the relative importance of various sources of nutrients in the watershed. It is our understanding that the City has contracted with Camp Dresser & McKee, Inc. to model the Trout River sub-basin using the Watershed Management Model to develop discharge and pollutant load information as part of an update to your local Master Stormwater Management Plan. The TMDL reductions were not based on model estimated loads and/or concentrations. As discussed in Chapter 5, the assimilative capacity was based upon relationships developed between water quality measurements taken in the middle Trout River WBID.

4) In general, the draft TMDLs for the Arlington, Ortega, and Trout Rivers apply to WBIDs that border the LSJR and are highly tidally influenced. Measured water quality values within these tidally influenced WBIDs are most likely reflective of water quality within the LSJR itself. The reductions needed to protect these WBIDs from impairment will be addressed through the LSJR Mainstem Basin Management Action Plan (BMAP). The projects identified in the Mainstem BMAP will be protective of water quality in these WBIDs and will achieve the specific load reductions.

Department Response: Once statistical relationships between dissolved oxygen, chlorophyll, and nutrients were developed for these impaired waters, the percent reduction in nitrogen required under the Mainstem nutrient TMDL was the starting point to assess whether designated uses would be restored within these tributaries with implementation of nitrogen reductions. In the case of the middle Trout River WBID, the TMDL also requires reduction in the total phosphorus. We concur that implementation of projects that improve water quality in the mainstem of the St. Johns River should also benefit the Arlington and portions of the Ortega and Trout Rivers due to tidal exchange. In recognition of this, the TMDL rule language has been written to allow for recognition of projects that provide co-benefits to the mainstem and each individual tributary.

Please contact Dr. Wayne Magley, at 850/245-8463, if you have any further questions.

Sincerely,

Jan Mandrup-Poulsen, Administrator Watershed Evaluation and TMDL Section

ec: Wayne Magley, DEP Lisa Sterling, CDM Jeff Martin, DEP



Florida Department of Environmental Protection Division of Water Resource Management Bureau of Watershed Management 2600 Blair Stone Road, Mail Station 3565 Tallahassee, Florida 32399-2400