FLORIDA DEPARTMENT OF ENVIRONMENTAL PROTECTION

Division of Environmental Assessment and Restoration, Bureau of Watershed Restoration

NORTHEAST DISTRICT • LOWER ST. JOHNS BASIN

## **Final TMDL Report**

# Fecal Coliform TMDL for New Castle Creek, WBID 2235

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#### **Websites**

# Florida Department of Environmental Protection, Bureau of Watershed Management

Total Maximum Daily Load (TMDL) Program http://www.dep.state.fl.us/water/tmdl/index.htm Identification of Impaired Surface Waters Rule http://www.dep.state.fl.us/legal/Rules/shared/62-303/62-303.pdf STORET Program http://www.dep.state.fl.us/water/storet/index.htm 2008 305(b) Report http://www.dep.state.fl.us/water/docs/2008 Integrated Report.pdf Criteria for Surface Water Quality Classifications http://www.dep.state.fl.us/water/wqssp/classes.htm Basin Status Report for the Lower St. Johns Basin http://www.dep.state.fl.us/water/tmdl/stat\_rep.htm Water Quality Assessment Report for the Lower St. Johns Basin http://www.dep.state.fl.us/water/tmdl/stat\_rep.htm

#### **U.S. Environmental Protection Agency**

Region 4: Total Maximum Daily Loads in Florida http://www.epa.gov/region4/water/tmdl/florida/ National STORET Program http://www.epa.gov/storet/

### Chapter 1: INTRODUCTION

#### 1.1 Purpose of Report

This report presents the Total Maximum Daily Load (TMDL) for fecal coliform for New Castle Creek in the North Mainstem Planning Unit of the Lower St. Johns Basin. The creek has been verified as impaired for fecal coliform, and was included on the Verified List of impaired waters for the Lower St. Johns Basin that was adopted by Secretarial Order in May 2004. This TMDL establishes the allowable loadings to New Castle Creek that would restore the waterbody so that it meets its applicable water quality criterion for fecal coliform.

#### **1.2 Identification of Waterbody**

New Castle Creek, located in Duval County in northeast Florida, drains an area of approximately 1.09 square miles (mi<sup>2</sup>). A direct tributary of the St. Johns River (**Figures 1.1** and **1.2**), the creek is approximately 1.43 miles long and is a second-order stream. The New Castle Creek watershed is located within the Jacksonville city limits, in the central portion of Duval County, on the south side of the St. Johns River, about 4.6 miles east of Interstate 95 (I-95). Additional information about the creek's hydrology and geology are available in the Basin Status Report for the Lower St. Johns Basin (Florida Department of Environmental Protection [Department], 2004).

For assessment purposes, the Department has divided the St. Johns River Basin into water assessment polygons with a unique **w**ater**b**ody **id**entification (WBID) number for each watershed or stream reach. New Castle Creek consists of one segment, WBID 2235 (**Figure 1.2**), which this TMDL addresses.

New Castle Creek is part of the North Mainstem Planning Unit. Planning units are groups of smaller watersheds (WBIDs) that are part of a larger basin unit, in this case the Lower St. Johns Basin. The North Mainstem Planning Unit consists of 51 WBIDs. **Figure 1.3** shows the location of these WBIDs, New Castle Creek's location in the planning unit, and a list of the other WBIDs in the planning unit.

#### Figure 1.1. Location of New Castle Creek, WBID 2235, and Major Geopolitical Features in the Lower St. Johns Basin





#### Figure 1.2. Overview of the New Castle Creek Watershed, WBID 2235

WBID	Waterbody Name
2181	Dunn Creek
2191	Broward River
2203	Trout River
2204	Terrapin Creek
2207	Blockhouse Creek
2210	West Branch
2213P	Ortega River
2220	Nine Mile Creek
2220	Ribault River
2224	Sherman Creek
2227	Moncrief Creek
2220	Sixmile Beach
2232	Long Branch
2235 2235	New Castle Creek
2233	Little Sixmile Creek
2230	Strawberry Crook
2239	Groopfield Crock
2240	Greenheid Creek
2244	
2240	Cip Llouge Creek
2248	Gin House Creek
2249B	Orte an Diver
2249A	Urtega River
2252	Hogan Creek
2254	Red Bay Branch
2256	Deer Creek
2257	McCoy Creek
2262	
2265B	Potsburg Creek
2266	Hopkins Creek
2270	Hogpen Creek
2278	Silversmith Creek
2280	Big Fishwier Creek
2282	Wills Branch
2284	Little Potsburg Creek
2287	Miller Creek
2297	Craig Creek
2299	Open Creek
2304	Miramar Creek
2306	New Rose Creek
2316	VVIIIIamson Creek
2321	Unristopher Branch
2322	Butcher Pen Creek
2324	Fishing Creek
2326	Goobys
2351	Julington Creek
2356	Big Davis Creek
2361	Deep Bottom Creek
2365	Durbin Creek
2370	Oldfield Creek
2381	Cormorant Branch
2385	Mandarin Drain

#### Figure 1.3. WBIDs in the North Mainstem Planning Unit

![](_page_11_Figure_3.jpeg)

#### **1.3 Background**

This report was developed as part of the Department's watershed management approach for restoring and protecting state waters and addressing TMDL Program requirements. The watershed approach, which is implemented using a cyclical management process that rotates through the state's 52 river basins over a 5-year cycle, provides a framework for implementing the TMDL Program–related requirements of the 1972 federal Clean Water Act and the 1999 Florida Watershed Restoration Act (FWRA) (Chapter 99-223, Laws of Florida).

A TMDL represents the maximum amount of a given pollutant that a waterbody can assimilate and still meet water quality standards, including its applicable water quality criteria and its designated uses. TMDLs are developed for waterbodies that are verified as not meeting their water quality standards. They provide important water quality restoration goals that will guide restoration activities.

This TMDL Report will be followed by the development and implementation of a Basin Management Action Plan, or BMAP, to reduce the amount of fecal coliform that caused the verified impairment of New Castle Creek. These activities will depend heavily on the active participation of the St. Johns River Water Management District (SJRWMD), the city of Jacksonville, Jacksonville Electric Authority (JEA), local businesses, and other stakeholders. The Department will work with these organizations and individuals to undertake or continue reductions in the discharge of pollutants and achieve the established TMDLs for impaired waterbodies.

## Chapter 2: DESCRIPTION OF WATER QUALITY PROBLEM

#### 2.1 Statutory Requirements and Rulemaking History

Section 303(d) of the federal Clean Water Act requires states to submit to the U.S. Environmental Protection Agency (EPA) a list of surface waters that do not meet applicable water quality standards (impaired waters) and establish a TMDL for each pollutant causing impairment of these waters on a schedule. The Department has developed such lists, commonly referred to as 303(d) lists, since 1992. The list of impaired waters in each basin, referred to as the Verified List, is also required by the FWRA (Subsection 403.067[4], Florida Statutes [F.S.]), and the state's 303(d) list is amended annually to include basin updates.

Florida's 1998 303(d) list included 55 waterbodies and 277 parameters in the Lower St. Johns River Basin. However, the FWRA (Section 403.067, F.S.) stated that all previous Florida 303(d) lists were for planning purposes only and directed the Department to develop, and adopt by rule, a new science-based methodology to identify impaired waters. After a long rule-making process, the Environmental Regulation Commission adopted the new methodology as Chapter 62-303, Florida Administrative Code (F.A.C.) (Identification of Impaired Surface Waters Rule, or IWR) in April 2001; the rule was amended in 2006 and 2007.

#### 2.2 Information on Verified Impairment

The Department used the IWR to assess water quality impairments in New Castle Creek and has verified that the creek is impaired for fecal coliform based on data in the Department's IWR database. **Tables 2.1** through **2.3** provide summary results for fecal coliform data for the verification period (which for Group 2 waters was January 1, 1996, to June 30, 2003), by month, season, and year, respectively. There is an 88.46 percent overall exceedance rate for fecal coliform in New Castle Creek during the verified period. There are a total of 26 samples, ranging from 140 counts per 100 milliliters (counts/100mL) to 11,000 counts/100mL, with 23 samples exceeding the fecal coliform criterion.

Exceedances occurred in all months in which samples were collected, except for March. March and April were the only months where 100 percent exceedance rates did not occur (**Table 2.1**). All months had exceedance rates greater than 66 percent except March (no exceedances). The sample size for each month is small, with all months having 4 or fewer samples, making interpretation difficult.

When aggregating data by season, the summer and fall seasons had the highest percentages of exceedances, as well as the highest (summer) and the lowest (fall) amounts of rainfall (**Table 2.2**). Due to small sample size, it is not clear whether exceedances are directly associated with rainfall events, nonpoint sources, point sources, or seasonal variation.

The yearly data show that exceedance rates started declining in the last 2 years of the verified period (**Table 2.3**). Sample size is small, ranging from 1 to 7 samples per year, making it difficult to verify potential trends. From the data that are available, the trend shows that exceedances remained at 100 percent until 2002 (which was 71.43 percent).

Historical fecal coliform data were collected at 2 sites during the verified period (January 1, 1996, to June 30, 2003). Most of the samples were taken by the city of Jacksonville and the rest were gathered by the Department. **Section 5.1** discusses sampling stations further.

Month	N	Minimum	Maximum	Median	Mean	Number of Exceedances	% Exceedances	Mean Precipitation
January	3	600	3,000	800	1,467	3	100.00	2.55
February	1	1,700	1,700	1,700	1,700	1	100.00	2.82
March	2	140	290	215	215	0	0.00	4.26
April	3	260	3,000	800	1,353	2	66.67	2.79
May	2	800	1,100	950	950	2	100.00	1.61
June	2	1,300	2,200	1,750	1,750	2	100.00	6.18
July	2	2,400	11,000	6,700	6,700	2	100.00	6.36
August	3	1,700	3,600	3,500	2,933	3	100.00	6.97
September	2	5,000	7,000	6,000	6,000	2	100.00	10.01
October	4	2,400	6,400	3,700	4,050	4	100.00	3.74
November	1	10,100	10,100	10,100	10,100	1	100.00	1.81
December	1	532	532	532	532	1	100.00	3.46

# Table 2.1. Summary of Fecal Coliform Data by Month for the VerifiedPeriod (January 1, 1996–June 30, 2003)

Coliform counts are #/100mL.

Exceedances represent values above 400 counts/100mL.

Mean precipitation is for Jacksonville International Airport (JIA) in inches.

# Table 2.2. Summary of Fecal Coliform Data by Season for the VerifiedPeriod (January 1, 1996–June 30, 2003)

						Number of	%	Mean
Season	N	Minimum	Maximum	Median	Mean	Exceedances	Exceedances	Precipitation
Winter	6	140	3,000	700	1,088	4	66.67	3.21
Spring	7	260	3,000	1,100	1,351	6	85.71	3.53
Summer	7	1,700	11,000	3,600	4,886	7	100.00	7.78
Fall	6	532	10,100	3,700	4,472	6	100.00	3.00

Coliform counts are #/100mL.

Exceedances represent values above 400 counts/100mL.

Mean precipitation is for JIA in inches.

\*Represents a monthly average for that season.

# Table 2.3. Summary of Fecal Coliform Data by Year for the VerifiedPeriod (January 1, 1996–June 30, 2003)

Year	N	Minimum	Maximum	Median	Mean	Number of Exceedances	% Exceedances	Mean Precipitation
1996	1	5,000	5,000	5,000	5,000	1	100.00	5.05
1997	1	5,000	5,000	5,000	5,000	1	100.00	4.77
1998	3	1,300	2,400	2,400	2,033	3	100.00	4.73
1999	4	800	2,400	1,400	1,500	4	100.00	3.54
2000	4	800	10,100	3,250	4,350	4	100.00	3.31
2001	4	532	7,000	2,350	3,058	4	100.00	4.1
2002	7	260	11,000	2,200	3,479	5	71.43	4.56
2003	2	140	800	470	470	1	50.00	3.71

Table represents years for which data exist.

Coliform counts are #/100mL.

Exceedances represent values above 400 counts/100mL.

Total precipitation is for JIA in inches.

## Chapter 3. DESCRIPTION OF APPLICABLE WATER QUALITY STANDARDS AND TARGETS

#### 3.1 Classification of the Waterbody and Criteria Applicable to the TMDL

Florida's surface waters are protected for five designated use classifications, as follows:

Class I	Potable water supplies
Class II	Shellfish propagation or harvesting
Class III	Recreation, propagation, and maintenance of a healthy, well- balanced population of fish and wildlife
Class IV	Agricultural water supplies
Class V	Navigation, utility, and industrial use (there are no state waters currently in this class)

New Castle Creek is a Class III fresh waterbody, with a designated use of recreation, propagation, and maintenance of a healthy, well-balanced population of fish and wildlife. The Class III water quality criterion applicable to the impairment addressed by this TMDL is for fecal coliform.

#### 3.2 Applicable Water Quality Standards and Numeric Water Quality Target

#### 3.2.1 Fecal Coliform Criterion

Numeric criteria for bacterial quality are expressed in terms of fecal coliform bacteria concentrations. The water quality criterion for protection of Class III waters, as established by Chapter 62-302, F.A.C., states the following:

#### Fecal Coliform Bacteria:

The most probable number (MPN) or membrane filter (MF) counts per 100 mL of fecal coliform bacteria shall not exceed a monthly average of 200, nor exceed 400 in 10 percent of the samples, nor exceed 800 on any one day.

The criterion states that monthly averages shall be expressed as geometric means based on a minimum of 10 samples taken over a 30-day period. However, there are insufficient data (fewer than 10 samples in a given month) available to evaluate the geometric mean criterion for fecal coliform bacteria. Therefore, the criterion selected for the TMDL is that samples shall not exceed 400 counts/100mL.

### Chapter 4: ASSESSMENT OF SOURCES

#### 4.1 Types of Sources

An important part of the TMDL analysis is the identification of pollutant source categories, source subcategories, or individual sources of pollutants in the watershed and the amount of pollutant loading contributed by each of these sources. Sources are broadly classified as either "point sources" or "nonpoint sources." Historically, the term "point sources" has meant discharges to surface waters that typically have a continuous flow via a discernable, confined, and discrete conveyance, such as a pipe. Domestic and industrial wastewater treatment facilities (WWTFs) are examples of traditional point sources. In contrast, the term "nonpoint sources" was used to describe intermittent, rainfall-driven, diffuse sources of pollution associated with everyday human activities, including runoff from urban land uses, agriculture, silviculture, and mining; discharges from failing septic systems; and atmospheric deposition.

However, the 1987 amendments to the Clean Water Act redefined certain nonpoint sources of pollution as point sources subject to regulation under the EPA's National Pollutant Discharge Elimination System (NPDES) Program. These nonpoint sources included certain urban stormwater discharges, such as those from local government master drainage systems, construction sites over five acres, and a wide variety of industries (see **Appendix A** for background information on the federal and state stormwater programs).

To be consistent with Clean Water Act definitions, the term "point source" will be used to describe traditional point sources (such as domestic and industrial wastewater discharges) **AND** stormwater systems requiring an NPDES stormwater permit when allocating pollutant load reductions required by a TMDL (see **Section 6.1**). However, the methodologies used to estimate nonpoint source loads do not distinguish between NPDES stormwater discharges and non-NPDES stormwater discharges, and as such, this source assessment section does not make any distinction between the two types of stormwater.

#### 4.2 Potential Sources of Coliform in the New Castle Creek Watershed

#### 4.2.1 Point Sources

There are currently no facilities with a permit to discharge wastewater in the New Castle Creek watershed (**Figure 4.1**).

#### **Municipal Separate Storm Sewer System Permittees**

The city of Jacksonville and Florida Department of Transportation (FDOT) District 2 are copermittees for a Phase I NPDES municipal separate storm sewer system (MS4) permit (FLS000012) that covers the New Castle Creek watershed. FDOT and the cities of Jacksonville, Neptune Beach, and Atlantic Beach share responsibility for the permit.

**Figure 4.2** shows the stormwater infrastructure of the watershed. Outfalls represent points where a conveyance of stormwater discharges into a separate stormwater system through a

#### Figure 4.1. Permitted Discharge Facilities in the New Castle Creek Watershed, WBID 2235

![](_page_18_Figure_2.jpeg)

![](_page_19_Figure_1.jpeg)

![](_page_19_Figure_2.jpeg)

channelized or natural waterway. Inlets are a component of the stormwater system located along the curbed edge of paved surfaces or the low point of an area to provide for the collection of stormwater runoff, access for inspection and maintenance, pipe junctions, sediment traps, or conflicts with other utilities (K. Grable, personal communication, October 16, 2008). In the New Castle Creek watershed, there are 54 outfalls and 357 inlets.

#### 4.2.2 Land Uses and Nonpoint Sources

Additional coliform loadings to New Castle Creek are generated from nonpoint sources in the watershed. Potential nonpoint sources of coliform include loadings from surface runoff, agriculture, wildlife, pets, leaking or overflowing sewer lines, and leaking septic tanks.

#### Land Uses

The spatial distribution and acreage of different land use categories were identified using the 2004 land use coverage contained in the Department's Geographic Information System (GIS) library, initially provided by the SJRWMD. Land use categories and acreages in the watershed were aggregated using the Level 3 codes tabulated in **Table 4.1**. Figure 4.3 shows the principal Level 2 land uses in the watershed.

The New Castle Creek watershed covers 1.09 mi<sup>2</sup>. As **Table 4.1** shows, most of the land in the watershed is residential (581 acres, or 83 percent). Natural areas add up to about 37 acres, or 5 percent, with the rest consisting of human-impacted areas (81 acres, or approximately 12 percent).

Level 3 Land Use			% of
Code	Attribute	Acres	Total
1100	Residential, low density– less than 2 dwelling units/acre	11.95	1.71
1200	Residential, medium density– 2-5 dwelling units/acre	518.70	74.25
1300	Residential, high density– 6 or more dwelling units/acre	50.53	7.23
1400	Commercial and services	33.40	4.78
1700	Institutional	28.65	4.10
4340	Upland mixed coniferous/hardwood	5.84	0.84
5100	Streams and waterways	5.41	0.77
5300	Reservoirs-pits, retention ponds, dams	2.38	0.34
6420	Saltwater marshes	5.44	0.78
6440	Emergent aquatic vegetation	4.96	0.71
6460	Mixed scrub-shrub wetland	14.90	2.13
8140	Roads and highways (divided 4-lanes with medians)	16.47	2.36
	TOTAL:	698.63	98.29

# Table 4.1. Classification of Land Use Categories in the New CastleCreek Watershed, WBID 2235

#### Figure 4.3. Principal Level 2 Land Uses in the New Castle Creek Watershed, WBID 2235, in 2004

![](_page_21_Figure_2.jpeg)

#### **Population**

According to the U.S Census Bureau, census block population densities in the New Castle Creek watershed in the year 2000 ranged from 0 to 10,000 people/mi<sup>2</sup>, with an average of 4,043 people/mi<sup>2</sup> in the watershed (**Figure 4.4**). Based on this average, the estimated population in the New Castle Creek watershed is 4,407 (**Table 4.2**). The Census Bureau reports that for all of Duval County, the total population for 2000 was approximately 780,000, with 329,778 housing units and an average occupancy rate of 92.1 percent (303,747 units). For all of Duval County, the Bureau reported a housing density of 426 houses per square mile. This places Duval County seventh in housing densities and population in Florida (U.S. Census Bureau Website, 2005). The estimated average housing density in New Castle Creek is 1,488 houses per square mile, based on population, which is higher than that of Duval County.

#### **Septic Tanks**

Approximately 78 percent of Duval County residences are connected to a wastewater treatment plant, while the rest use septic tanks (PBS&J, 2007; and Florida Department of Health [FDOH] Website, 2006b). FDOH reports that as of fiscal year 2003–04, there were 88,834 permitted septic tanks in Duval County and for fiscal years 1993 to 2004, 5,479 permits for repairs were issued, or an average of approximately 457 repairs annually countrywide.

The Department obtained septic tank repair permit data from JEA for its service area, which includes the New Castle Creek watershed. Based on the data provided by JEA, which are more watershed specific than the countywide FDOH data, 17 septic tank repair permits were issued from 1990 to 2006. This equates to an average of 1.29 permits issued per year, which can be rounded up to 1 (to allow for those septic tanks where failures may not be known or have not been repaired). With 2 septic tank failures, 2.72 people per household, and using an estimate of 70 gallons/day/person (EPA, 2001), a potential loading of 1.44 x  $10^{10}$  fecal colonies/day is derived. **Table 4.3** shows this estimation.

The data provided by JEA also include areas serviced by a WWTF and areas where large numbers of failing septic tanks are present. **Figure 4.5** presents this information in map form. A small portion of the New Castle Creek watershed is in a septic tank phase-out area (an area with the highest priority to be sewered due to its high septic tank failure rates). **Figure 4.5** shows that these areas include 0.01 mi<sup>2</sup> (17,324.13 square meters) at the southeastern tip of the New Castle Creek watershed (depicted in yellow). The New Castle Creek watershed is serviced by the Arlington East WWTF (in the northern portion of the WBID) and the Monterey WWTF (in the southern region of the WBID).

#### Figure 4.4. Population Density in the New Castle Creek Watershed, WBID 2235, in 2000

![](_page_23_Figure_2.jpeg)

# Table 4.2. Estimated Average Household Size in the New Castle CreekWatershed, WBID 2235

	Number of		Number of		
Household Size	Households	% of Total	People		
1-person household	304	18.74	304		
2-person household	549	33.82	1,097		
3-person household	316	19.47	947		
4-person household	281	17.35	1,125		
5-person household	115	7.11	577		
6-person household	42	2.62	255		
7-or-more-person household	14	0.89	101		
TOTAL:	1,622	100.00	4,407		
AVERAGE HOUSEHOLD SIZE: 2.72					

\*Individual values for number of people per household size are rounded to the nearest whole number, while total number of people remains based on unrounded values.

# Table 4.3. Estimated Daily Fecal Coliform Loading from Failed SepticTanks in the New Castle Creek Watershed, WBID 2235

Estimated Population Density (persons/mi <sup>2</sup> )	WBID Area (mi <sup>2</sup> )	Estimated Population in Watershed	Estimated Number of Tank Failures <sup>1</sup>	Estimated Load from Failed Tanks <sup>2</sup>	Gallons/ Person/ Day <sup>2</sup>	Estimated Number of Persons per Household <sup>3</sup>	Estimated Daily Load from Failing Tanks	Estimated Annual Load from Failing Tanks
4,043	1.09	4,407	2	1.00 x 10 <sup>4</sup> /mL	70	2.75	1.44 x 10 <sup>10</sup>	5.26 x 10 <sup>12</sup>

<sup>1</sup> Based on septic tank repair permits issued in the watershed from March 1990 to April 2004 (FDOH and JEA information); see text. <sup>2</sup> EPA, 2001.

<sup>3</sup> From U.S. Census Bureau; see **Table 4.2** for more information on this estimate.

#### Figure 4.5. Septic Tank Repair Permits Issued for the New Castle Creek Watershed, 1990–2004

![](_page_25_Figure_2.jpeg)

#### 4.3 Source Summary

#### 4.3.1 Agriculture

Based on Level 3 land use data, there are no agricultural areas in the New Castle Creek watershed.

#### 4.3.2 Pets

Pets, especially dogs, may be having an impact on the waterbody. The Department has been unable to obtain data on the number of dogs in the area; however, estimates can be made using literature-based values of dog ownership rates (**Table 4.4**). For example, using household-to-dog ratio estimates from the American Veterinary Medical Association (AVMA), the approximate loading is  $4.70 \times 10^{12}$  organisms/day.

# Table 4.4. Estimated Loading from Dogs in the New Castle CreekWatershed, WBID 2235

Estimated Number of Households in WBID 2235	Estimated Dog: Household Ratio <sup>1</sup>	Estimated Number of Dogs in WBID 2235	Estimated Fecal Coliform (counts/dog/day <sup>2</sup> (	Estimated Fecal Coliform (counts/day(	Estimated Fecal Coliform (counts/year)
1,622	0.58	941	5 x 10 <sup>9</sup>	4.70 x 10 <sup>12</sup>	1.72 x 10 <sup>15</sup>

<sup>1</sup> From the AVMA Website, which states the original source to be the *U.S. Pet Ownership and Demographics Sourcebook*, 2002.

<sup>2</sup> EPA, 2001.

#### 4.3.3 Leaking or Overflowing Wastewater Collection Systems

As noted previously, about 78 percent of households in Duval County are connected to a wastewater facility. Assuming 1,622 homes in the watershed, with 2.72 people per home, and a 70-gallon-per-person-per-day discharge, and also assuming that the countywide average of 78 percent of households connected to a WWTF applies in the New Castle Creek watershed, a daily flow of approximately  $9.12 \times 10^5$  liters (L) is transported through the collection system. The EPA (Davis, 2002) suggests that a 5 percent leakage rate from collection systems is a realistic estimate. Based on this rate and EPA values for fecal coliform in raw sewage, the potential loading of fecal coliform from leaking sewer lines is  $2.28 \times 10^{12}$  counts/day (**Table 4.5**).

# Table 4.5. Estimated Loading from Wastewater Collection Systems in<br/>the New Castle Creek Watershed, WBID 2235

Estimated					
Number of	Estimated			Estimated	Estimated
Homes on	Daily	Daily		Fecal	Fecal
Central Sewer in	Flow	Leakage	Raw Sewage	Coliform	Coliform
WBID 2235	(L)	(L) _	(counts/100mL)	(counts/day)	(counts/year)

**Table 4.6** summarizes the various estimates from potential sources. It is important to note that this is not a complete list (wildlife, for example, is missing) and represents estimates of potential loadings. Proximity to the waterbody, rainfall frequency and magnitude, and temperature (viability of fecal coliform) are just a few of the factors that could influence and determine the actual loadings from these sources that reach New Castle Creek.

#### Table 4.6. Summary of Estimated Potential Coliform Loading from Various Sources in the New Castle Creek Watershed, WBID 2235

Source	Fecal Coliform (counts/day)	Fecal Coliform (counts/year)
Septic Tanks	1.44 x 10 <sup>10</sup>	5.26 x 10 <sup>12</sup>
Pets	4.70 x 10 <sup>12</sup>	1.72 x 10 <sup>15</sup>
Collection Systems	2.28 x 10 <sup>12</sup>	8.32 x 10 <sup>14</sup>

## Chapter 5: DETERMINATION OF ASSIMILATIVE CAPACITY

#### 5.1 Determination of Loading Capacity

The methodology used for this TMDL was the "percent reduction" methodology. The Department generally prefers to use the load duration curve or "Kansas" method for coliform TMDLs, but this method could not be used because there are no stream gauging stations on New Castle Creek. To determine the TMDL, the percent reduction that would be required for each of the exceedances to meet the applicable criterion was determined, and the median value of all of these reductions for fecal coliform determined the overall required reduction, and is therefore the TMDL.

#### 5.1.1 Data Used in the Determination of the TMDL

There are six sampling stations on New Castle Creek, two of which have historical fecal coliform observations: New Castle Creek at Greenfern Lane (STORET ID: 21FLA20030804) and New Castle Creek at old Ft. Caroline Road (STORET ID: 21FLJXWQARL6) (Figure 5.1). The Department monitors the Greenfern Lane station, which is located towards the southern end of the creek. It was sampled three times in 2002. The city of Jacksonville monitors the Caroline Road station, which is located approximately 0.6 miles above the Greenfern Lane station. The city of Jacksonville maintained routine (mostly quarterly) sampling from 1991 to 2006 at its station (21FLJXWQARL6). Table 5.1 shows data collection information for each station, while Figure 5.1 displays the locations of the sample sites, and Table 5.2 shows observed historical data analysis. Appendix B contains all of the historical fecal coliform observations from each site for the planning and verified periods for the Lower St. Johns Basin. Figure 5.2 shows these observations over time.

The Greenfern Lane station had fewer exceedances (66.67 percent) than the station at Caroline Road (94.55 percent) (**Table 5.2**), indicating that the downstream station has much fewer exceedances than the upstream station (not considering decay rates and the settling out that may occur in that 0.6 mile). However, 52 more samples were taken at the Caroline Road station; the vast difference in sample size makes it impossible to compare the number of exceedances.

			Years With	
Station	STORET ID	Monitoring Agency	Data	Ν
New Castle Creek at Greenfern Lane	21FLA 20030804	Department (Northeast District)	2002	3
New Castle Creek at Old Ft. Caroline Road	21FLJXWQARL6	City of Jacksonville	1991–2006	55

#### Table 5.1. Sampling Station Summary for New Castle Creek, WBID 2235

![](_page_29_Figure_1.jpeg)

Figure 5.1. Historical Sample Sites in New Castle Creek, WBID 2235

# Table 5.2. Statistical Summary of All Historical Data for New CastleCreek, WBID 2235

								%
Station	STORET ID	Ν	Minimum	Maximum	Median	Mean	Exceedances	Exceedances
New Castle Creek at Greenfern Lane	21FLA 20030656	3	290	3,600	2,200	2,030	2	66.67
New Castle Creek at Old Ft. Caroline Road	21FLJXWQARL6	55	140	160,000	2,400	6,777	52	94.55

Coliform concentrations are counts/100mL.

#### Figure 5.2. Historical Fecal Coliform Observations for New Castle Creek, WBID 2235

![](_page_30_Figure_5.jpeg)

#### 5.1.2 TMDL Development Process

Due to the lack of supporting flow information, a simple calculation was performed to determine the needed reduction. Exceedances of the state criterion were compared with the criterion of 400 counts/100mL. For each individual exceedance, an individual required reduction was calculated using the following:

#### [(observed value) – (state criterion)] x 100 (observed value)

23

After the individual results were calculated, the median of the individual values was calculated, which is 84 percent. This means that in order to meet the state criterion of 400 counts/100mL, an 84 percent reduction in current loading is necessary, and is therefore the TMDL for New Castle Creek. **Table 5.3** shows the annual summaries of exceedances used to determine the TMDL by year, and **Table 5.4** shows the individual exceedances used in calculating the TMDL for New Castle Creek.

Year	Ν	Minimum	Maximum	Median	Mean
1991	4	3,000	160,000	10,500	46,000
1992	4	800	13,000	5,150	6,025
1993	4	2,400	8,000	2,700	3,950
1994	4	170	16,000	1,900	4,993
1995	2	1,300	2,200	1,750	1,750
1996	1	5,000	5,000	5,000	5,000
1997	1	5,000	5,000	5,000	5,000
1998	3	1,300	2,400	2,400	2,033
1999	4	800	2,400	1,400	1,500
2000	4	800	10,100	3,250	4,350
2001	4	532	7,000	2,350	3,058
2002	7	260	11,000	2,200	3,479
2003	4	140	4,800	1,100	1,785
2004	4	800	7,500	2,800	3,475
2005	4	700	14,000	2,205	4,778
2006	4	800	9,000	2,700	3,800

# Table 5.3. Annual Summary of Fecal Coliform Exceedances Used ToDevelop the TMDL for New Castle Creek, WBID 2235

Coliform counts are #/100mL and represent years for which exceedances exist.

# Table 5.4. Calculation of Reductions for the Fecal Coliform TMDL forNew Castle Creek, WBID 2235

Sample Date	Location	Observed Value (Exceedance) (#/100mL)	Required Reduction (%)
1/23/1991	New Castle Creek at Old Ft. Caroline Road	3,000	86.67
5/14/1991	New Castle Creek at Old Ft. Caroline Road	16,000	97.50
8/13/1991	New Castle Creek at Old Ft. Caroline Road	160,000	99.75
10/28/1991	New Castle Creek at Old Ft. Caroline Road	5,000	92.00
1/27/1992	New Castle Creek at Old Ft. Caroline Road	800	50.00
5/11/1992	New Castle Creek at Old Ft. Caroline Road	8,000	95.00
7/21/1992	New Castle Creek at Old Ft. Caroline Road	13,000	96.92
10/21/1992	New Castle Creek at Old Ft. Caroline Road	2,300	82.61
1/27/1993	New Castle Creek at Old Ft. Caroline Road	2,400	83.33
4/19/1993	New Castle Creek at Old Ft. Caroline Road	2,400	83.33
7/21/1993	New Castle Creek at Old Ft. Caroline Road	3,000	86.67
10/20/1993	New Castle Creek at Old Ft. Caroline Road	8,000	95.00
1/18/1994	New Castle Creek at Old Ft. Caroline Road	800	50.00
7/25/1994	New Castle Creek at Old Ft. Caroline Road	3,000	86.67
10/11/1994	New Castle Creek at Old Ft. Caroline Road	16,000	97.50
1/24/1995	New Castle Creek at Old Ft. Caroline Road	1,300	69.23

Sample		Observed Value (Exceedance)	Required Reduction
Date	Location	(#/100mL)	(%)
4/24/1995	New Castle Creek at Old Ft. Caroline Road	2,200	81.82
10/23/1996	New Castle Creek at Old Ft. Caroline Road	5,000	92.00
9/17/1997	New Castle Creek at Old Ft. Caroline Road	5,000	92.00
6/3/1998	New Castle Creek at Old Ft. Caroline Road	1,300	69.23
7/21/1998	New Castle Creek at Old Ft. Caroline Road	2,400	83.33
10/27/1998	New Castle Creek at Old Ft. Caroline Road	2,400	83.33
1/20/1999	New Castle Creek at Old Ft. Caroline Road	800	50.00
5/3/1999	New Castle Creek at Old Ft. Caroline Road	1,100	63.64
8/31/1999	New Castle Creek at Old Ft. Caroline Road	1,700	76.47
10/13/1999	New Castle Creek at Old Ft. Caroline Road	2,400	83.33
1/12/2000	New Castle Creek at Old Ft. Caroline Road	3,000	86.67
4/12/2000	New Castle Creek at Old Ft. Caroline Road	800	50.00
8/28/2000	New Castle Creek at Old Ft. Caroline Road	3,500	88.57
11/27/2000	New Castle Creek at Old Ft. Caroline Road	10,100	96.04
2/20/2001	New Castle Creek at Old Ft. Caroline Road	1,700	76.47
4/24/2001	New Castle Creek at Old Ft. Caroline Road	3,000	86.67
9/10/2001	New Castle Creek at Old Ft. Caroline Road	7,000	94.29
12/3/2001	New Castle Creek at Old Ft. Caroline Road	532	24.81
1/31/2002	New Castle Creek at Old Ft. Caroline Road	600	33.33
6/25/2002	New Castle Creek at Greenfern Lane	2,200	81.82
7/15/2002	New Castle Creek at Old Ft. Caroline Road	11,000	96.36
8/26/2002	New Castle Creek at Greenfern Lane	3,600	88.89
10/16/2002	New Castle Creek at Old Ft. Caroline Road	6,400	93.75
5/27/2003	New Castle Creek at Old Ft. Caroline Road	800	50.00
9/26/2003	New Castle Creek at Old Ft. Caroline Road	4,800	91.67
12/1/2003	New Castle Creek at Old Ft. Caroline Road	1,400	71.43
3/2/2004	New Castle Creek at Old Ft. Caroline Road	800	50.00
6/21/2004	New Castle Creek at Old Ft. Caroline Road	2,600	84.62
9/15/2004	New Castle Creek at Old Ft. Caroline Road	7,500	94.67
12/6/2004	New Castle Creek at Old Ft. Caroline Road	3,000	86.67
3/8/2005	New Castle Creek at Old Ft. Caroline Road	2,210	81.90
5/16/2005	New Castle Creek at Old Ft. Caroline Road	2,200	81.82
8/29/2005	New Castle Creek at Old Ft. Caroline Road	700	42.86
11/28/2005	New Castle Creek at Old Ft. Caroline Road	14,000	97.14
2/14/2006	New Castle Creek at Old Ft. Caroline Road	800	50.00
4/26/2006	New Castle Creek at Old Ft. Caroline Road	3,000	86.67
8/22/2006	New Castle Creek at Old Ft. Caroline Road	9,000	95.56
11/1/2006	New Castle Creek at Old Ft. Caroline Road	2,400	83.33
	MEDIAN:	2,500	84.00

#### 5.1.3 Critical Conditions/Seasonality

Exceedances in New Castle Creek cannot be associated with flows, as there are no flow data available in the watershed. Therefore, the effects of flow under various conditions cannot be determined or be considered as a critical condition.

**Appendix B** provides historical fecal coliform observations in New Castle Creek. Coliform data are presented by month, season, and year to determine whether certain patterns are evident in the dataset.

It is very difficult to evaluate possible patterns among months due to the small sample sizes; the range in monthly observations for fecal coliform varies from 1 to 4 in a given month (**Table 2.1**). March had no exceedances, while April had the second lowest exceedance rate, at 66.67 percent. All other months had a 100 percent exceedance rate.

Grouping observations by season increased sample sizes for statistical comparison, as seen in **Table 2.2**, but sample size is still relatively small (between 4 and 7 samples). Summer and fall demonstrated the greatest percentage of exceedances (100 percent) while winter exhibited the lowest (66.67 percent). **Appendix E** presents comparisons by station and season. A nonparametric test (Kruskal-Wallis) was applied to the fecal coliform dataset to determine whether there were significant differences among months or seasons. The analysis indicated that there were significant differences (at an  $\alpha$  level of 0.05) between fecal coliform observations versus season (**Appendix C**) and versus month (**Appendix D**).

A factor that could contribute to these monthly or seasonal differences is the pattern of rainfall. Rainfall records for JIA (**Appendix F** illustrates rainfall from 1990 to 2008) were used to determine rainfall amounts associated with individual sampling dates. Rainfall recorded on the day of sampling (1D), the cumulative total for the day of and the previous 2 days (3D), the cumulative total for the day of and the previous 6 days (7D), and the cumulative total for the day of sampling and 29 days prior (30D) were all paired with the respective coliform observation based on date.

A Spearman correlation matrix was generated that summarized the simple correlation coefficients between the various rainfall and coliform values (**Appendix G**). The simple correlations (r values in the Spearman correlation table) between both fecal coliform and the various rainfall totals were all positive, suggesting that as rainfall (and possible runoff) increased, so did the number of coliform. Simple linear regressions were performed between coliform observations and rainfall totals to determine whether any of the relationships were significant at an  $\alpha$  level of 0.05. The r<sup>2</sup> values between fecal coliform and all of the various precipitation intervals were not significant, except for the day of sampling precipitation (see **Appendix H**). This may reflect the response associated with rainfall events in a small watershed.

A table of historical monthly average rainfall (**Appendix I**) indicates that monthly rainfall totals increase in June, peak in September, and by October return to the levels observed in February and March. **Appendix J** includes a graph of annual rainfall from 1955 to 2008, versus the long-term average (52.47 inches) over this period. The years 1996 to 1998 had above-average rainfall; while 1999 to 2001 were below average; and 2002, 2004, and 2005 were again above average; followed by a drop in rainfall in 2006. Data exceedances occur almost all of the time, making it difficult to correlate them with rainfall patterns.

**Table 2.3** shows that exceedances do not appear to follow the same pattern as rainfall. From 1996 to 2001, every sample was an exceedance (100 percent exceedances), and then the exceedances started tampering off in 2002 and were lowest in 2003 (50 percent). There was above-average rainfall in 1996, 1997, 1998, and 2002, while 1999, 2000, 2001, and 2003 were below average. The lowest percentage of exceedances was observed in 2003 (50 percent), a below-average rainfall year, but a trend with rainfall cannot accurately be established. Most likely this is attributable to the small sample size, between 1 and 5 samples per year.

As no flow data were available, hydrologic conditions were analyzed using rainfall. A loading curve-type chart that would normally be applied to flow events was created using precipitation

data from JIA from 1990 to 2004. The chart was divided in the same manner as if flow were being analyzed, where extreme precipitation events represent the upper percentiles (0 to 5<sup>th</sup> percentile), followed by large precipitation events (5<sup>th</sup> to 10<sup>th</sup> percentile), medium precipitation events (10<sup>th</sup> to 40<sup>th</sup> percentile), small precipitation events (40<sup>th</sup> to 60<sup>th</sup> percentile), and no recordable precipitation events (60<sup>th</sup> to 100<sup>th</sup> percentile). Three-day (the day of and 2 days prior to sampling) precipitation accumulations were used in the analysis (**Figure 5.3**).

Fecal coliform exceedances occurred over all hydrologic conditions for which data exist. The lowest percentage of exceedances occurred during the period of medium precipitation events (87.50 percent). The highest percentage (100 percent) occurred after extreme and large precipitation events; however, these periods also had the fewest samples. There were only 4 samples collected within 3 days of an extreme precipitation event and 1 within 3 days of a large precipitation event.

It is difficult to draw conclusions with so few samples representing extreme and large precipitation events; however, if these 2 events are excluded due to small sample size, exceedances appear to decrease as precipitation amounts decrease, indicating that nonpoint sources are probably a major contributing factor. There are still large numbers of exceedances occurring when there is little or no precipitation. These exceedances at baseflow can be attributed to ground water contributions from failed septic tanks and/or leaking collection systems. A pattern could become clearer if more samples were collected, especially following extreme and large rainfall events. **Table 5.5** summarizes data and hydrologic conditions. **Figure 5.3** shows the same data visually.

Precipitation Event	Event Range (inches)	Total Samples	Number of Exceedances	% Exceedance	Number of Nonexceedances	% Nonexceedances
Extreme	>2.1"	4	4	100.00	0	0.00
Large	1.33" - 2.1"	1	1	100.00	0	0.00
Medium	0.18" - 1.33"	16	14	87.50	2	14.29
Small	0.01" - 0.18"	12	11	91.67	1	8.33
None/ No Measurable	<0.01"	25	24	96.00	1	4.00

#### Table 5.5. Summary of Fecal Coliform Data by Hydrologic Condition

![](_page_35_Figure_1.jpeg)

Figure 5.3. Fecal Coliform Data by Hydrologic Condition Based on Rainfall

### Chapter 6: DETERMINATION OF THE TMDL

#### 6.1 Expression and Allocation of the TMDL

The objective of a TMDL is to provide a basis for allocating acceptable loads among all of the known pollutant sources in a watershed so that appropriate control measures can be implemented and water quality standards achieved. A TMDL is expressed as the sum of all point source loads (wasteload allocations, or WLAs), nonpoint source loads (load allocations, or LAs), and an appropriate margin of safety (MOS), which takes into account any uncertainty concerning the relationship between effluent limitations and water quality:

#### $\mathsf{TMDL} = \sum \mathsf{WLAs} + \sum \mathsf{LAs} + \mathsf{MOS}$

As discussed earlier, the WLA is broken out into separate subcategories for wastewater discharges and stormwater discharges regulated under the NPDES Program:

#### $\textbf{TMDL} \cong \sum \textbf{WLAs}_{wastewater} + \sum \textbf{WLAs}_{NPDES \; Stormwater} + \sum \textbf{LAs} + \textbf{MOS}$

It should be noted that the various components of the revised TMDL equation may not sum up to the value of the TMDL because (a) the WLA for NPDES stormwater is typically based on the percent reduction needed for nonpoint sources and is also accounted for within the LA, and (b) TMDL components can be expressed in different terms (for example, the WLA for stormwater is typically expressed as a percent reduction, and the WLA for wastewater is typically expressed as mass per day).

WLAs for stormwater discharges are typically expressed as "percent reduction" because it is very difficult to quantify the loads from MS4s (given the numerous discharge points) and to distinguish loads from MS4s from other nonpoint sources (given the nature of stormwater transport). The permitting of stormwater discharges also differs from the permitting of most wastewater point sources. Because stormwater discharges cannot be centrally collected, monitored, and treated, they are not subject to the same types of effluent limitations as wastewater facilities, and instead are required to meet a performance standard of providing treatment to the "maximum extent practical" through the implementation of best management practices (BMPs).

This approach is consistent with federal regulations (40 CFR § 130.2[I]), which state that TMDLs can be expressed in terms of mass per time (e.g., pounds per day), toxicity, or **other appropriate measure**. The TMDL for New Castle Creek is expressed in terms of both counts/100mL and percent reduction, and represents the maximum daily fecal coliform load the creek can assimilate and maintain the fecal coliform criterion (**Table 6.1**).

#### Table 6.1. TMDL Components for New Castle Creek, WBID 2235

			WLA		IA	
WBID	Parameter	TMDL (colonies/100mL)	Wastewater (colonies/day)	NPDES Stormwater	(% Reduction)	MOS
2235	Fecal Coliform	400	N/A	84%	84%	Implicit

#### 6.2 Load Allocation

The load allocation (LA) for nonpoint sources is an 84 percent reduction of instream fecal coliform concentrations. It should be noted that the load allocation includes loading from stormwater discharges that are not part of the NPDES Stormwater Program.

#### 6.3 Wasteload Allocation

#### 6.3.1 NPDES Wastewater Discharges

There are currently no permitted NPDES discharges into New Castle Creek. Any future discharge permits issued in the watershed will be required to meet the state Class III criterion for fecal coliform as well as the TMDL value, and therefore will not be allowed to exceed 200 counts/100mL as a monthly average, 400 counts/100mL in more than 10 percent of the samples, or 800 counts/100mL at any given time.

#### 6.3.2 NPDES Stormwater Discharges

The wasteload allocation (WLA) for the city of Jacksonville and FDOT's MS4 permit is to address anthropogenic sources in the watershed to result in an 84 percent reduction of instream fecal coliform concentrations. It should be noted that any MS4 permittee is only responsible for reducing the loads associated with stormwater outfalls that it owns or otherwise has responsible control over, and it is not responsible for reducing other nonpoint source loads in its jurisdiction.

While the LA and WLA for fecal coliform are expressed as the percent reductions needed to attain the applicable Class III criterion, it is the combined reductions from both anthropogenic point and nonpoint sources that will result in the required reduction of instream fecal coliform concentrations. However, it is not the intent of these TMDLs to abate natural background conditions.

#### 6.4 Margin of Safety

Consistent with the recommendations of the Allocation Technical Advisory Committee (Department, 2001), an implicit MOS was used in the development of this TMDL by not allowing any exceedances of the state criterion, even though the actual criterion allows for 10 percent exceedances over the fecal coliform criterion of 400 counts/100mL.

## Chapter 7: NEXT STEPS: IMPLEMENTATION PLAN DEVELOPMENT AND BEYOND

#### 7.1 Basin Management Action Plan

Following the adoption of this TMDL by rule, the next step in the TMDL process is to develop an implementation plan, or BMAP, for the TMDL. The first BMAP for the tributaries to the Lower St. Johns River will address the 10 worst-case impairments in the 55 tributaries impaired for fecal coliform. Any future BMAPs will address additional subsets of the tributaries listed for fecal coliform.

In addition to addressing failing septic tanks, the BMAP may include some sort of public education program about pet waste cleanup. As **Table 4.4** shows, potential impacts from dogs in the watershed could be significant. If pet owners are educated on the potential impacts their pets are having on New Castle Creek, and they are inclined to take action, this could potentially decrease a source load. When considering the significance of seven-day rainfall, this could be a potentially significant load to the stream.

Through the implementation of projects, activities, and additional source assessments in the BMAP, stakeholders expect the following outcomes:

- Improved water quality trends in the tributaries of the Lower St. Johns River, which will also help improve water quality in the main stem of the river;
- Decreased loading of the target pollutant (fecal coliform);
- Enhanced public awareness of pollutant sources, pollutant impacts on water quality, and corresponding corrective actions;
- Enhanced understanding of basin hydrology, water quality, and pollutant sources; and
- The ability to evaluate management actions, estimate their benefits, and identify additional pollutant sources.

#### 7.1.1 Determination of Worst-Case WBIDs

The initial determination of the worst-case WBIDs uses a ranking method that establishes the severity of bacterial contamination based on the number of exceedances of fecal coliform colony counts—i.e., the number of total fecal coliform samples in a waterbody during the period of record to indicate how many samples are over 800, 5,000, and 10,000 colony counts. A combined rank is then created based on the number of exceedances in each category. The WBIDs are sorted from worst to best to provide a guideline for assessment priorities, with the worst-case waterbody ranked first. Future BMAPs will continue to address the worst-case waters first, using the ranking method.

#### 7.1.2 Identification of Probable Sources

#### **Tributary Pollutant Assessment Project**

Initial sampling for the study on the six initial WBIDs of highest concern began July 26, 2005, and was completed on February 1, 2006. The final deliverable (the *Tributary Pollutant Assessment Project Manual*) was submitted to JEA on June 1, 2006, and became available for public review and comment on June 16, 2006. Four types of fecal indicators (fecal coliform, *E. coli., Enterococci*, and coliphages) were studied. *Enterococcus faecalis* was also studied in an attempt to further identify potential sources of sewage, and samples were checked for human/ruminant primers.

The executive summary submitted to the Department by JEA and PBS&J is attached as **Appendix K**. The results of the study will be used to help guide the identification of restoration projects during BMAP development.

#### **Technical Reports**

In an effort to address the known impairments in the Lower St. Johns tributaries, the Department contracted with Post, Buckley, Schuh & Jernigan (PBS&J) to develop technical reports that describe and interpret the water quality, spatial, and geographic data from the Department, Duval County Health Department, city of Jacksonville, and JEA. The reports analyze the available data to identify the most probable sources of fecal coliform, which fall into five main categories, as follows: (1) stormwater, (2) onsite sewage treatment and disposal systems (OSTDS), (3) sewer infrastructure, (4) nonpoint sources such as pet waste, and (5) natural background such as wildlife. These reports were peer reviewed by technical stakeholders in the basin, who also provided additional input based on their knowledge of the tributaries.

#### 7.1.3 Issues To Be Addressed in Future Watershed Management Cycles

The BMAP process identified the following items that should be addressed in future watershed management cycles to ensure that future BMAPs use the most accurate information:

- 1. **Source Identification**—Sources of fecal coliform impairment are particularly difficult to trace. For this reason, the BMAP includes source identification studies as management actions.
- 2. **Septic Tanks**—The Department is implementing a study, Evaluation of Septic Tank Influences on Nutrient Loading to the Lower St. Johns River Basin and Its Tributaries, to better understand the nutrient and bacteria loading from septic tanks via ground water by monitoring conditions at representative sites. The study seeks to answer questions on potential OSTDS impacts and the attenuation of nitrogen, phosphorus, and bacteria (fecal coliform) by soil, under the range of conditions that represent typical OSTDS sites near impaired surface waters. It will also document the nutrients and bacteria in the receiving Lower St. Johns tributaries at each site. The results will provide information about the relative contribution of fecal coliform from septic tanks located near the impaired tributaries.
- 3. **GIS Information**—During the BMAP process, the available GIS data, which provide a basis for some of the source analyses, have improved. As more

information becomes available, the updated GIS database for the tributaries will be utilized to aid in source identification. This information will include determining the spatial locations for private wastewater systems and infrastructure, collecting jurisdictional or systemwide programs and activities on a WBID scale for future reporting and assessment, and systematically updating all GIS information databases used to compile the BMAP.

#### 7.1.4 BMAP Implementation

The BMAP requires that all stakeholders implement their projects to achieve reductions as soon as practicable. However, the full implementation of the BMAP will be a long-term process. While some of the projects and activities in the BMAP are recently completed or currently ongoing, several projects will require more time to design, secure funding, and construct. Although funding the projects could be an issue, funding limitations do not affect the requirement that every entity must implement the activities listed in the BMAP.

Since BMAP implementation is a long-term process, the TMDL targets established for the Lower St. Johns Basin will not be achieved in the next five years. It may take even longer for the tributaries to respond to reduced loadings and fully meet applicable water quality standards. Regular follow-up and continued coordination and communication among the stakeholders will be essential to ensure the implementation of management strategies and the assessment of their incremental effects. Any additional management actions required to achieve TMDLs, if necessary, will be developed as part of BMAP follow-up.

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### **Appendices**

#### Appendix A: Background Information on Federal and State Stormwater Programs

In 1982, Florida became the first state in the country to implement statewide regulations to address the issue of nonpoint source pollution by requiring new development and redevelopment to treat stormwater before it is discharged. The Stormwater Rule, as authorized in Chapter 403, F.S., was established as a technology-based program that relies on the implementation of BMPs that are designed to achieve a specific level of treatment (i.e., performance standards) as set forth in Chapter 62-40, F.A.C. In 1994, the Department's stormwater treatment requirements were integrated with the stormwater flood control requirements of the state's water management districts, along with wetland protection requirements, into the Environmental Resource Permit regulations

Chapter 62-40, F.A.C., also requires the water management districts to establish stormwater pollutant load reduction goals (PLRGs) and adopt them as part of a Surface Water Improvement and Management (SWIM) plan, other watershed plan, or rule. Stormwater PLRGs are a major component of the load allocation part of a TMDL.

In 1987, the U.S. Congress established Section 402(p) as part of the federal Clean Water Act Reauthorization. This section of the law amended the scope of the federal NPDES stormwater permitting program to designate certain stormwater discharges as "point sources" of pollution. The EPA promulgated regulations and began implementing the Phase I NPDES Stormwater Program in 1990. These stormwater discharges include certain discharges that are associated with industrial activities designated by specific standard industrial classification (SIC) codes, construction sites disturbing 5 or more acres of land, and master drainage systems of local governments with a population above 100,000, which are better known as MS4s. However, because the master drainage systems of most local governments in Florida are interconnected, the EPA has implemented Phase 1 of the MS4 permitting program on a countywide basis, which brings in all cities (incorporated areas), Chapter 298 urban water control districts, and FDOT throughout the 15 counties meeting the population criteria. The EPA authorized the Department to implement the NPDES Stormwater Program (except for tribal lands) in October 2000.

An important difference between the federal and the state's stormwater/environmental resource permitting programs is that the NPDES Program covers both new and existing discharges, while the state's program focuses on new discharges only. Additionally, Phase II of the NPDES Program, implemented in 2003, expands the need for these permits to construction sites between 1 and 5 acres, and to local governments with as few as 10,000 people. The revised rules require that these additional activities obtain permits by 2003. While these urban stormwater discharges are now technically referred to as "point sources" for the purpose of regulation, they are still diffuse sources of pollution that cannot be easily collected and treated by a central treatment facility, as are other point sources of pollution, such as domestic and industrial wastewater discharges. It should be noted that all MS4 permits issued in Florida include a reopener clause that allows permit revisions to implement TMDLs when the implementation plan is formally adopted.

#### Appendix B: Historical Fecal Coliform Observations in New Castle Creek, WBID 2235

Waterbody	WBID	Sample Date	Station	Location	Value (#/100mL)	Remark Code
New Castle Creek	2235	3/28/2002	21FLA 20030656	New Castle Creek at Greenfern Lane	290	
New Castle Creek	2235	6/25/2002	21FLA 20030656	New Castle Creek at Greenfern Lane	2,200	
New Castle Creek	2235	8/26/2002	21FLA 20030656	New Castle Creek at Greenfern Lane	3,600	
New Castle Creek	2235	1/23/1991	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	3,000	
New Castle Creek	2235	5/14/1991	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	16,000	
New Castle Creek	2235	8/13/1991	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	160,000	
New Castle Creek	2235	10/28/1991	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	5,000	
New Castle Creek	2235	1/27/1992	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	800	
New Castle Creek	2235	5/11/1992	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	8,000	
New Castle Creek	2235	7/21/1992	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	13,000	
New Castle Creek	2235	10/21/1992	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	2,300	
New Castle Creek	2235	1/27/1993	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	2,400	
New Castle Creek	2235	4/19/1993	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	2,400	
New Castle Creek	2235	7/21/1993	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	3,000	
New Castle Creek	2235	10/20/1993	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	8,000	
New Castle Creek	2235	1/18/1994	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	800	
New Castle Creek	2235	4/12/1994	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	170	
New Castle Creek	2235	7/25/1994	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	3,000	
New Castle Creek	2235	10/11/1994	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	16,000	
New Castle Creek	2235	1/24/1995	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	1,300	
New Castle Creek	2235	4/24/1995	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	2,200	
New Castle Creek	2235	10/23/1996	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	5,000	
New Castle Creek	2235	9/17/1997	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	5,000	
New Castle Creek	2235	6/3/1998	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	1,300	
New Castle Creek	2235	7/21/1998	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	2,400	
New Castle Creek	2235	10/27/1998	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	2,400	
New Castle Creek	2235	1/20/1999	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	800	
New Castle Creek	2235	5/3/1999	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	1,100	
New Castle Creek	2235	8/31/1999	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	1,700	
New Castle Creek	2235	10/13/1999	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	2,400	
New Castle Creek	2235	1/12/2000	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	3,000	
New Castle Creek	2235	4/12/2000	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	800	
New Castle Creek	2235	8/28/2000	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	3,500	
New Castle Creek	2235	11/27/2000	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	10,100	K
New Castle Creek	2235	2/20/2001	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	1,700	
New Castle Creek	2235	4/24/2001	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	3,000	
New Castle Creek	2235	9/10/2001	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	7,000	
New Castle Creek	2235	12/3/2001	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	532	
New Castle Creek	2235	1/31/2002	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	600	
New Castle Creek	2235	4/10/2002	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	260	
New Castle Creek	2235	7/15/2002	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	11,000	
New Castle Creek	2235	10/16/2002	21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road	6,400	
New Castle Creek	2235	3/5/2003	ARL6	New Castle Creek at Ft. Caroline Hills Rd	140	
New Castle Creek	2235	5/27/2003	ARL6	New Castle Creek at Ft. Caroline Hills Rd	800	

#### FINAL TMDL Report: Lower St. Johns Basin, New Castle Creek, WBID 2235, Fecal Coliform, June 2009

		Sample	_		Value	Remark
Waterbody	WBID	Date	Station	Location	(#/100mL)	Code
New Castle Creek	2235	9/26/2003	ARL6	New Castle Creek at Ft. Caroline Hills Rd	4,800	
New Castle Creek	2235	12/1/2003	ARL6	New Castle Creek at Ft. Caroline Hills Rd	1,400	
New Castle Creek	2235	3/2/2004	ARL6	New Castle Creek at Ft. Caroline Hills Rd	800	
New Castle Creek	2235	6/21/2004	ARL6	New Castle Creek at Ft. Caroline Hills Rd	2,600	
New Castle Creek	2235	9/15/2004	ARL6	New Castle Creek at Ft. Caroline Hills Rd	7,500	
New Castle Creek	2235	12/6/2004	ARL6	New Castle Creek at Ft. Caroline Hills Rd	3,000	
New Castle Creek	2235	3/8/2005	ARL6	New Castle Creek at Ft. Caroline Hills Rd	2,210	
New Castle Creek	2235	5/16/2005	ARL6	New Castle Creek at Ft. Caroline Hills Rd	2,200	
New Castle Creek	2235	8/29/2005	ARL6	New Castle Creek at Ft. Caroline Hills Rd	700	
New Castle Creek	2235	11/28/2005	ARL6	New Castle Creek at Ft. Caroline Hills Rd	14,000	
New Castle Creek	2235	2/14/2006	ARL6	New Castle Creek at Ft. Caroline Hills Rd	800	
New Castle Creek	2235	4/26/2006	ARL6	New Castle Creek at Ft. Caroline Hills Rd	3,000	
New Castle Creek	2235	8/22/2006	ARL6	New Castle Creek at Ft. Caroline Hills Rd	9,000	
New Castle Creek	2235	11/1/2006	ARL6	New Castle Creek at Ft. Caroline Hills Rd	2,400	

\*Deleted blank result entries and dups.

COJ data from PBS&J.

Values that exceed the state criterion of 400 counts/100mL.

Remark Code: K – Less than.

**NOTE:** Some samples were seen as duplicates (i.e., same date and location) and were averaged, per the IWR, for TMDL determination. **Appendix B** includes all data contained in the IWR database. For this reason, some discrepancies may exist between **Appendix B** and the tables in the text.

## Appendix C: Kruskal–Wallis Analysis of Fecal Coliform Observations versus Season in New Castle Creek, WBID 2235

Categorical values encountered during processing are: SEASON\$ (4 levels) FALL, SPRING, SUMMER, WINTER

Kruskal-Wallis One-Way Analysis of Variance for 58 cases Dependent variable is FECAL Grouping variable is SEASON\$

 Group
 Count
 Rank Sum

 FALL
 14
 516.500

 SPRING
 15
 363.000

 SUMMER
 15
 605.000

 WINTER
 14
 226.500

Kruskal-Wallis Test Statistic = 19.140 Probability is 0.000 assuming Chi-square distribution with 3 df

#### Appendix D: Kruskal–Wallis Analysis of Fecal Coliform Observations versus Month in New Castle Creek, WBID 2235

Kruskal-Wallis One-Way Analysis of Variance for 58 cases Dependent variable is FECAL Grouping variable is MONTH

Group Count Rank Sum

1	8	156.000
2	2	30.500
3	4	40.000
4	7	138.500
5	5	154.000
6	3	70.500
7	5	207.500
8	6	216.500
9	4	181.000
10	8	322.000
11	3	135.500
12	3	59.000

Kruskal-Wallis Test Statistic = 26.174 Probability is 0.006 assuming Chi-square distribution with 11 df

![](_page_47_Figure_1.jpeg)

#### Appendix E: Chart of Fecal Coliform Observations by Season and Station in New Castle Creek, WBID 2235

Of 58 cases, 1 was excluded by making graph range less than data range.

	20000	
	19000	
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R	12000	
$\overline{\mathbf{A}}$	11000	
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		200° 1200
		P J'
		STATIONS

![](_page_48_Figure_2.jpeg)

	FAI	
--	-----	--

- □ SPRING
- □ WINTER

Station	Location
21FLA 20030804	New Castle Creek at Greenfern Lane
21FLJXWQARL6	New Castle Creek at Old Ft. Caroline Road

#### Appendix F: Chart of Rainfall for JIA, 1990–2008

![](_page_49_Figure_2.jpeg)

#### Appendix G: Spearman Correlation Matrix Analysis for Precipitation and Fecal Coliform in New Castle Creek, WBID 2235

Spearman correlation matrix

	YEAR	MONTH	DAY	FECALS	V1D_PREC
YEAR	1.000				
MONTH	-0.079	1.000			
DAY	0.150	-0.074	1.000		
FECALS	-0.194	0.210	-0.031	1.000	
V1D_PREC	0.118	-0.181	-0.088	-0.021	1.000
V3D_PREC	0.016	0.153	0.259	0.192	0.562
V7D_PRE	-0.030	0.226	0.044	0.247	0.311
MONTH_PR	-0.096	0.127	0.150	0.125	0.197
	V3D_PREC	V7D_PRE	MONTH_PR	_	
V3D_PREC	1.000				
V7D_PRE	0.634	1.000			
MONTH_PR	0.335	0.454	1.000		

![](_page_50_Figure_4.jpeg)

Number of observations: 68

#### Appendix H: Analysis of Fecal Coliform Observations and Precipitation in New Castle Creek, WBID 2235

#### FECAL COLIFORM DATA VERSUS DAY OF SAMPLING PRECIPITATION

Dep Var: FECAL N: 58 Multiple R: 0.991 Squared multiple R: 0.982

Analysis of Variance

Source	Sum-of-Squares	df	Mean-Square	F-ratio	Р
V1D_PREC	2.44136E+10	19	1.28493E+09	111.471	0.000
Error	4.38026E+08	38	1.15270E+07		

![](_page_51_Figure_6.jpeg)

![](_page_51_Figure_7.jpeg)

*** WARN	NG ***		
Case	7 is an outlier	(Studentized Residual =	3.346)
Durbin-Wa First Order	tson D Statistic Autocorrelation	1.755 0.122	

### FECAL COLIFORM DATA VERSUS DAY OF SAMPLING AND 2 DAYS PRIOR PRECIPITATION

Dep Var: FECAL N: 58 Multiple R: 0.043 Squared multiple R: 0.002

Adjusted squared multiple R: 0.000 Standard error of estimate: 21046.661

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	6082.779	3095.475	0.000		1.965	0.054
V3D_PREC	699.229	2175.130	0.043	1.000	0.321	0.749

Analysis of Variance

First Order Autocorrelation

	Source	Sum-of-Squares	df	Mean-Square	F-ratio	Р
	Regression	4.57757E+07	1	4.57757E+07	0.103	0.749
	Residual	2.48059E+10	56	4.42962E+08		
*** WAR	NING ***					
Case	3 is an outlier	(Studentized Resid	lual =	42.622)		
Case	16 has large lev	erage (Leverage =	0.646	5)		
Durbin-V	Vatson D Statistic	1.860				

0.069

![](_page_52_Figure_8.jpeg)

![](_page_52_Figure_9.jpeg)

## FECAL COLIFORM DATA VERSUS DAY OF SAMPLING AND 6 DAYS PRIOR PRECIPITATION

Dep Var: FECAL N: 58 Multiple R: 0.028 Squared multiple R: 0.001

Adjusted squared multiple R: 0.000 Standard error of estimate: 21057.832

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	6993.571	3539.117	0.000		1.976	0.053
V7D_PRE	-311.341	1487.067	-0.028	1.000	-0.209	0.835

Analysis of Variance

	Source	Sum-of-Squares	df	Mean-Square	F-ratio	Р
Reg	ression	1.94374E+07	1	1.94374E+07	0.044	0.835
R	esidual	2.48322E+10	56	4.43432E+08		
*** WARNING	G ***					
Case 3	3 is an out	lier (Studentized Res	sidual = 4	0.239)		
Case 1	6 has larg	e leverage (Leverage =	0.284)			
Case 3	9 has larg	e leverage (Leverage =	0.271)			
Durbin-Watso First Order A	on D Statis utocorrelat	tic 1.878 ion 0.060				

![](_page_53_Figure_7.jpeg)

![](_page_53_Figure_8.jpeg)

## FECAL COLIFORM DATA VERSUS DAY OF SAMPLING AND 29 DAYS PRIOR PRECIPITATION

Dep Var: FECAL N: 58 Multiple R: 0.075 Squared multiple R: 0.006

Adjusted squared multiple R: 0.000 Standard error of estimate: 21196.137

Effect	Coefficient	Std Error	Std Coef	Tolerance	t	P(2 Tail)
CONSTANT	5201.079	4951.888	0.000		1.050	0.298
V7D_PRE	-825.058	1792.273	-0.074	0.697	-0.460	0.647
MONTH_PR	521.043	999.823	0.084	0.697	0.521	0.604

Analysis of Variance

Sou	Source Sum-of-Squa		uares df		Mean-Square	F-ratio	P
Regression 1.41453E			E+08	2	7.07264E+07	0.157	0.855
Residual 2.47			E+10	55	4.49276E+08		
*** WARNIN	NG ***						
Case	3 is an o	outlier (S	udentized Res	idual = 4	40.169)		
Case	16 has la	arge leverage	(Leverage =	0.288)			
Case	39 has la	arge leverage	(Leverage =	0.333)			
Durbin-Wate	son D Sta	atistic 1	.913				
First Order /	Autocorre	elation 0.	043				

#### Plot of residuals against predicted values

![](_page_54_Figure_8.jpeg)

Year	Jan	Feb	Mar	Apr	May	Jun	Jul	Διια	Sen	Oct	Nov	Dec	Annual Total
1955	3.1	2.46	1.66	1.5	4.51	2.7	5.53	3.85	10.6	5.36	1.9	0.2	43.33
1956	2.9	2.94	0.81	2.33	3.98	7.87	8.25	5.24	2.89	13.4	0.4	0	51.08
1957	0.3	1.69	3.87	1.61	5.25	7.1	12.3	3.3	8.33	3.5	1.6	1.3	50.18
1958	3.4	3.74	3.38	8.24	3.79	3.96	4.37	4.67	4.75	5.07	2	2.8	50.14
1959	3	5.22	9.75	2.65	9.2	2.94	4.51	2.86	5.67	3.12	2.2	1	52.08
1960	2.1	5.17	6.94	3.54	1.18	4.7	16.2	6.5	8.57	2.95	0.1	1.5	59.45
1961	2.9	4.85	1.17	4.16	3.06	5.27	3.48	10.6	1.02	0.27	0.9	0.5	38.15
1962	2.2	0.52	3.1	2.36	1.12	8.22	6.31	10.1	4.37	1.13	2.1	2.5	43.9
1963	5.4	6.93	2.23	1.75	1.74	12.5	6.47	4.95	4.88	1.53	2.7	3.6	54.66
1964	7.3	6.55	1.76	4.65	4.8	4.67	6.12	5.63	10.3	5.09	3.3	4.8	65.03
1965	0.7	5.5	3.91	0.95	0.94	9.79	2.71	9.58	11	1.75	1.9	3.8	52.47
1966	4.6	5.97	0.71	2.25	10.4	7.74	11.1	3.88	5.94	1.38	0.2	1.1	55.3
1967	3.1	4.35	0.81	2	1.18	12.9	5.22	12.3	1.8	1.13	0.2	4.7	49.68
1968	0.8	3.05	1.2	0.99	2.17	12.3	6.84	16.2	2.68	5.09	1.3	1.1	53.72
1969	0.8	3.39	4.23	0.34	3.78	5.12	5.89	15.1	10.3	9.81	4.6	3.9	67.26
1970	4.2	8.85	9.98	1.77	1.84	2.65	7.6	11	3.2	3.95	0	1.6	56.55
1971	2	2.55	2.41	4.07	1.9	5.52	5.07	12.8	4.17	6.46	0.8	5.9	53.69
1972	5.8	3.48	4.43	2.98	8.26	6.75	3.15	9.76	2.6	4.46	4.2	1.4	57.29
1973	4.6	5.07	10.2	11.6	5.33	4.1	5.45	7.49	7.86	4.08	0.4	4.3	70.57
1974	0.3	1.28	3.47	1.53	4.14	5.53	9.83	11.2	8.13	0.34	1	1.7	48.52
1975	3.5	2.58	2.46	5.78	7	5.21	6.36	6.23	5.24	3.63	0.4	1.8	50.15
1976	2.3	1.05	3.41	0.63	10	4.26	5.41	6.37	8.56	1.63	2.4	4.8	50.87
1977	3	3.24	1.03	1.76	3.07	2.65	1.97	7.26	7.45	1.68	3.1	3.4	39.56
1978	4.6	4.17	2.83	2.24	9.18	2.62	6.67	2.39	4.4	1.26	0.8	1.8	43.04
1979	6.3	3.75	1	4.18	7.54	5.91	4.67	4.78	17.8	0.25	3.6	2	61.76
1980	2.6	1.06	6.83	3.91	3.02	4.59	5.29	3.97	3.03	2.69	2.3	0.2	39.53
1981	0.9	4.53	5.41	0.32	1.48	3.31	2.46	6.47	1.22	1.35	4.9	3.4	35.77
1982	3	1.67	4.26	3.6	3.55	8.06	3.81	6.93	9.32	3.37	1.9	2	51.52
1983	7.2	4.27	8.46	4.65	1.38	6.86	6.11	4.63	4.61	4.29	3.3	6.4	62.19
1984	2.1	4.67	5.77	3.14	1.46	4.76	6.01	3.78	12.3	1.53	3.3	0.1	48.96
1985	1.1	1.45	1.26	2.76	2.08	3.71	6.33	8.93	16.8	8.34	2.1	3.6	58.39
1986	4.2	4.72	5.44	0.93	2.13	2.53	3.27	9.6	1.99	1.8	2.9	4.7	44.1
1987	4.1	6.47	6.27	0.14	0.75	4.18	4.4	4.48	7.13	0.3	5	0.2	43.39
1988	6.4	6.08	2.65	3.44	1.35	3.71	4.5	8.48	16.4	2.35	4.3	1.1	60.68
1989	1.7	1.77	2.14	2.79	1.55	3.66	8.98	9.16	14.4	1.39	0.5	3.4	51.45
1990	1.8	4.07	1.59	1.34	0.18	1.59	6.53	3.81	2.6	4.54	1.2	1.9	31.2
1991	10	1.52	7.33	6.31	9.35	11.7	15.9	3.48	6.2	6.36	0.7	0.6	79.63
1992	5.8	2.64	4.09	5.33	5.97	7.04	3.32	10.8	7.33	8.34	1.9	0.7	63.18
1993	3.9	2.89	5.98	0.85	1.6	2.52	7.54	2.96	7.6	8.84	3.6	1.9	50.12
1994	6.6	0.92	2.14	1.51	3.15	14	8.26	3.29	9.79	10.2	3.5	3.9	67.26
1995	1.9	2.07	3.67	1.77	1.77	5.35	9.45	9.93	5.41	3.53	3.2	2.2	50.25
1996	1.1	1.11	6.83	2.85	0.72	11.4	4.2	7.83	8.49	11.5	1.4	3.2	60.63
1997	2.9	1.28	1.84	4.56	3.43	6.33	7.69	8.24	3.97	4.84	2.4	9.8	57.27
1998	3.5	11.1	2.64	4.71	0.96	2.95	7.29	10.1	7.65	3.01	2.4	0.4	56.72

#### Appendix I: Monthly and Annual Precipitation from JIA, 1955–2008

Year	Jan	Feb	Mar	Apr	Мау	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual Total
1999	4.6	1.7	0.4	1.92	1.02	7.75	3.56	3.51	13	3.24	0.8	0.9	42.44
2000	2.8	1.17	1.79	2.6	1.15	2.43	5.69	7.38	11.6	0.23	1.6	1.4	39.77
2001	0.9	0.68	5.48	0.62	2.56	5.59	8.31	3.58	16	0.81	1.4	3.1	49.14
2002	4.5	0.82	4.38	2.41	0.47	6.24	7.8	8.14	9.31	2.58	2.7	5.4	54.72
2003	0.1	-	10.7	2.63	2.54	6.75	7.33	1.83	3.04	2.98	0.7	1.2	44.47
2004	1.6	4.47	1.36	2.02	1.24	17.2	8.6	9.85	16.3	1.32	2.9	2.7	69.47
2005	1.9	3.56	3.67	4.53	3.51	14.8	7.37	4.43	5.76	6.49	1.1	7.4	64.44
2006	2.30	3.91	0.68	1.22	2.01	7.25	3.97	7.08	4.55	1.81	0.39	2.90	38.07
2007	2.29	2.40	2.22	1.02	1.12	6.68	9.48	3.57	5.44	8.85	0.17	2.74	45.98
2008	2.63	5.22	3.50	2.34	0.66	8.21	8.83						31.39
AVG	3.21	3.54	3.81	2.82	3.29	6.37	6.55	6.99	7.43	3.87	1.98	2.62	52.47

Rainfall is in inches, and represents data from JIA.

![](_page_57_Figure_1.jpeg)

#### Appendix J: Annual and Monthly Average Precipitation at JIA

![](_page_58_Figure_1.jpeg)

#### Appendix K: Executive Summary of Tributary Pollution Assessment Project

**Note:** This appendix contains the executive summary of the Tributary Pollution Assessment Project (TPAP) submitted to the Department by JEA and PBS&J. The six phases detailed in the methodology development and evaluation section have already been completed as of the date of this TMDL. In place of the public workshop mentioned in the section describing Phase 6, the Tributary Pollution Assessment Manual was presented to the Jacksonville Waterways Commission on February 1, 2007.

The Tributary Pollution Assessment Project involves developing and evaluating a methodology for conducting tributary pollution assessments for listed water bodies in the Duval County area, as referenced in the Reasonable Assurance (RA) Plan. Duval County has approximately 100 tributary Water Body IDs (WBIDs), i.e. small to large tributaries of the St. Johns River, identified by the State. The RA Plan provides reasonable assurance that the fecal coliform levels of the 51 top-ranked WBIDs will be reduced sufficiently to restore them to their designated use for recreation. The 51 WBIDs are grouped into four priority groups in the RA Plan.

PBS&J was contracted by JEA to develop a methodology for conducting tributary pollution assessments for sources of fecal coliform contamination in the listed tributaries. This methodology will be field-verified by conducting sanitary surveys of selected tributary water body segments, and revised based on lessons learned from this process. The final product of this endeavor will be a *Tributary Pollution Assessment Manual* that can be used as a blueprint for conducting sanitary surveys.

The Tributary Pollution Assessment Project is a continuation of the effort started under the RA Plan. The RA Plan participants have been brought together to form the Tributary Assessment Team (TAT). The TAT will serve as an advisory committee to the PBS&J Project Team throughout the development of the *Tributary Pollution Assessment Manual*. The TAT is composed of representatives from:

- JEA
- City of Jacksonville Environmental Quality Division
- City of Jacksonville Public Works Department
- Duval County Health Department
- Florida Department of Environmental Protection
- St. Johns Riverkeeper
- Water and Sewer Expansion Authority
- US Army Corps of Engineers

Other representatives (from these and additional entities) may be included in the TAT activities in varying roles, as relevant.

Our approach for developing and evaluating a methodology for conducting tributary pollution assessments is divided into six major phases including:

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- 1) Pre-planning;
- 2) Planning;
- 3) Development of Tributary Pollution Assessment Manual;
- 4) Evaluation of Methodology/Manual by Conducting Sanitary Surveys;

5) Summary Report; and 6) Public Workshop.

The Pre-Planning phase (Phase I) entailed four main goals:

1) to obtain and review all documents included in the RA Plan;

2) to develop categories for tributary classification and categorize the 51 priority WBIDs;
3) to overlay each WBID onto land use, infrastructure, and historical sampling maps to begin assessing probable sources and migration pathways; and
4) to develop the *Draft Work Plan*.

The Planning phase (Phase II) begins with the organization and initial meeting of the Tributary Assessment Team (TAT) with the ultimate goal of finalizing the *Work Plan*.

The Development of the *Tributary Pollution Assessment Manual* phase (Phase III) primarily involves the formulation of the assessment methodology for each tributary category described in the Pre-Planning phase, the use of a decision tree to determine which assessment methodology corresponds to each of the highest-ranked WBIDs, and the establishment of a model monitoring plan for each tributary category. This phase will be completed upon submitting the *Manual* to the TAT for review.

The next phase, Evaluation of Methodology/Manual by Conducting Sanitary Surveys (Phase IV), entails field-verification of the methodology described in the *Draft Tributary Pollution Assessment Manual* for the highest ranked water bodies for each category (or as determined to ensure adequate geographical representation of the study area) and applying the results to recommend generic corrective actions and revise the methodology, if necessary. The outcome of this phase would be the *Tributary Pollution Assessment Manual*.

The final two phases, Summary Report (Phase V) and Public Workshop (Phase VI), would entail providing a summary of the results of the tributary pollution assessments, including a discussion of lessons learned and site-specific corrective actions, to JEA and presenting the results from the *Tributary Pollution Assessment Manual* to the public. The final phase would also include a written summary of public input received at the workshop.

For additional information, please contact: Don Deis, PBS&J Project Manager, at (904) 363-8442 or <u>drdeis@pbsj.com</u>.

![](_page_61_Picture_0.jpeg)

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